## HYDROGEOLOGIC ASSESSMENT REPORT

Dezel Ranch 660 Junction Plaza Clearlake, CA 95423 APNs 010-055-28, 010-055-29, 010-055-33, 010-055-37 and 010-055-38

#### **Prepared For:**

Mr. Simon Whetzel 660 Junction Plaza Clearlake, CA 95423

February 2, 2022 Revised October 7, 2022

#### **Prepared By:**

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Project No. 5185.01



February 2, 2022 Revised October 7, 2022

Mr. Simon Whetzel 660 Junction Plaza Clearlake, CA 95423

Re: Hydrogeologic Assessment Report

Dezel Ranch 660 Junction Plaza Clearlake, CA 95423

APN 010-055-28, 010-055-29, 010-055-33, 010-055-37 and 010-055-38

HES Project No. 5185.01

Mr. Simon Whetzel:

Hurvitz Environmental Services, Inc. (HES) is pleased to submit this Hydrogeologic Assessment Report for the above referenced property. HES prepared this Report in accordance with the Lake County Cannabis Ordinance. The purpose of this Report was to outline the site's proposed water usage rates and water conveyance systems as well as to evaluate whether or not the project water supply can adequately meet the proposed water demands without creating aquifer overdraft.

Based on the information and assessments contained herein, we conclude that the wells discharge capacity and rate of recharge are sufficient to sustainably provide for the projected annual water use at the site. The quantity of groundwater to be used for the project is unlikely to result in significant declines in regional groundwater availability or depletion of groundwater resources over time. The potential for the project water-use to cause well interference or impacts to creeks are also considered minimal.

We appreciate the opportunity to provide you with these services. Please do not hesitate to contact us at your convenience, should you have any questions or comments regarding this report or our recommendations.

Sincerely,

HURVITZ ENVIRONMENTAL SERVICES, INC

Lee S. Hurvitz, PG #7573 CHG #1015

Certified Hydrogeologist

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#### 1.0 INTRODUCTION AND SCOPE OF SERVICES

We understand that Simon Whetzel, (the applicant) is applying to Lake County for approval to develop approximately 8-acres (348,480 ft²) of outdoor cannabis cultivation and 0.92-acres (39,936ft²) of mixed light greenhouse cultivation (the project) at the property located at 660 Junction Plaza Clearlake, CA 95423, Assessor's Parcel Numbers (APN) 010-055-28, 010-055-29, 010-055-33, 010-055-37 and 010-055-38 (Site). According to the Lake County Cannabis Ordinance, development of property with the intent to cultivate cannabis requires a Hydrogeologic Assessment Report. Therefore, on behalf of the applicant Hurvitz Environmental Services (HES) conducted a Hydrogeologic Assessment Study and prepared this Report in accordance with the Lake County requirements.

This Hydrogeologic Assessment Report includes the following elements:

- Estimates of existing and proposed water uses for the property.
- Characterization of local geologic and hydrogeologic conditions including defining water sheds and sub-basins.
- Review and analysis of 6-hour well yield and recharge tests.
- Well Completion Report assessment.
- Discussion on proposed methods for water level and water usage monitoring.
- Aquifer storage and recharge assessment.
- Assess potential for well interference between the project well and neighboring wells and between the project well and nearby streams.

#### 2.0 SITE DESCRIPTION

The site is located in unincorporated Lake County, California, approximately 1 mile north of the City of Clearlake. Access to the site is obtained from Highway 53 south of Junction Plaza to the east on Ogulin Canyon Road (county road 215B) with private gravel and native soil surfaced access roads off of Ogulin Canyon Road (**Plate 1 – Site location Map**). The site consists of five parcels 010-055-28 (100.48-acres), 010-55-29 (17.66-acres), 010-055-33 (81.71-acres), 010-055-37 (153.92-acres) and 010-055-38 (77.81-acres) totaling approximately 431 acres (**Plate 2 – Assessor's Parcel Map**). Cultivation activities are only proposed on the three parcels APN 010-055-28, 010-055-29, and 010-055-33 (Cultivation Parcels – 199.85-acres).

The site lies in the Mayacama Mountains of the Coast Ranges to the east of Clearlake. Site landscapes range from flat grasslands to oak and chaparral forest. The topography in this location is undulating, consisting of a series of ridgelines and valleys with elevations that range from approximately 1,600 to 1,900 feet above mean sea level (MSL). (**Plate 3 – USGS Topographic Map**). The site has been improved with four groundwater wells and a small hunting cabin (**Plate 4 – Site Plan**). **Site photographs** are presented in **Appendix A** and the **Engineering Site Plans** are presented in **Appendix B**.

#### 2.1 USGS 7.5 MINUTE QUANDRANGLE MAP

HES reviewed the United States Geological Survey (USGS) Benmore Canyon 7.5-Minute Quadrangle Maps, 2015, (**Plate 3 – USGS Topographic Map**). The 431-acre site is separated by a north-south trending, watershed divide or ridge ridgeline that separates surface water flow patterns across the site. Surface water on the western portion of the site, including the cultivation parcels, drains west/southwest and into Burns Valley Creek and then Clearlake, while surface water from the eastern portion of the site drains east/southeast and into Phipps Creek and then Cache Creek.

Multiple ephemeral Class III watercourses drain southwesterly across the cultivation parcels and into an intermittent tributary to Burns Valley Creek. The intermittent tributary stream flows north-south, paralleling Ogulin Canyon Road, through all three cultivation parcels before discharging to Burns Valley Creek approximately 1-mile south of the site. Watersheds are discussed in section 2.3 below.

The peak elevation on the site is 1,960 feet MSL on the north boundary of parcel 010-055-38. The peak elevation on the cultivation parcels is 1,800 feet MSL, located on parcel 010-055-28. The lowest elevation onsite is approximately 1,560 feet MSL on the southside of parcel 010-055-28 where the unnamed intermittent watercourse flows south off the site (**Plate 3 – USGS Topographic Map**).

#### 2.2 GEOLOGICAL CONDITIONS

HES reviewed the Geologic Map and Structure Section of the Clear Lake Volcanics, Northern California<sup>1</sup>. According to the Map reviewed, the site lies within a geologic region characterized mostly by Quaternary deposits of the Cache Formation (Qtc) including siltstone, sandstone, conglomeritic sandstone and tuff. These rocks are overlain by younger alluvium (al), colluvium (co), and landslide (ls) deposits of Holocene age. In addition, Pleistocene terrace deposits of the Burns Valley (tb) are found in the canyons created by the unnamed tributary to Burns Valley in the western portion of the site. Also on the western side of the site are early basaltic rocks of Pleistocene and Pliocene age characterized as undivided flows pyroclastic and intrusive rock composed of olivine basalt and olivine basaltic andesite (beu). These volcanics were deposited over the Upper Cretaceous to Upper Jurassic Franciscan Complex (KFJ) basement rock which is a mixture of marine chert, greenstone, greywacke, shale and metamorphic rocks of blueschist grade. To the west of the site is the Cross Spring Fault Zone.

#### 2.3 REGIONAL GROUNDWATER

The western portion of the site including the cultivation areas are located within the Burns Valley-Frontal Clear Lake Sub-watershed (HUC 12 - 180201160309) and the east side of the site is located in the Grizzly Creek-North Fork Cache Creek Sub-watershed (HUC 12 – 180201160408). Water from the Burns Valley-Frontal Clear Lake Sub-watershed flows to Clearlake and water from the Grizzly Creek-North Fork Cache Creek Sub-watershed flows to the Central Valley. These watersheds are within the jurisdiction of the Central Valley Regional Water Quality Control Board. (**Plate 3 – USGS Topographic Map**).

The western side of the site, including the cultivation areas, is located within the Burns Valley Groundwater Basins as identified in the 2006 Lake County Groundwater Management Plan<sup>2</sup>, while the remainder of the site is located within the Clear Lake Cache Formation Groundwater Basins. The Burns Valley Basin is in the Shoreline Inventory Unit. The Franciscan Complex borders the Burns Valley Basin on the north, Clear Lake borders the basin on the west, and the Cache Formation borders the basin on the south and east. Water-bearing formations include the Quaternary alluvium and terrace deposits as well as recent basaltic volcanics.

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<sup>&</sup>lt;sup>1</sup> USGS 1995 Geologic Map and Structure section of the Clear Lake Volcanics, Northern California, B.C. Hearn, Jr, J.M. Donnelly-Nolan, and F.E. Goff.

<sup>&</sup>lt;sup>2</sup> Camp Dresser and McKee Inc. 2006. Lake County Groundwater Management Plan. Camp Dresser & McKee Inc., Sacramento, California. Report prepared for Lake County Flood Control and Watershed Protection District, Lake County, California.

#### 3.0 SITE DEVELOPMENT AND WATER USE

The proposed cultivation operation would be composed of 348,480 ft<sup>2</sup> outdoor canopy area and a 39,936 ft<sup>2</sup> canopy area in mixed light greenhouses. The proposed outdoor canopy area would be planted in early June and harvested throughout the month of October (~150-day cultivation season). Plants would be cultivated within the proposed mixed-light canopy area year-round, with minimal operations occurring during the months of January and February. The growing medium of the proposed outdoor canopy area will be native soil amended with compost. The proposed mixed-light canopy area would be located within gutter connected greenhouse structures, composted of steel frames and polycarbonate walls and roofs. The growing medium of the proposed mixed-light canopy area will be an imported organic soilless growing medium (composed mostly of composted forest material) in garden beds and nursery pots.

Irrigation for cultivation will be from two irrigation wells designated as irrigation wells A and B and located adjacent to the cultivation area as shown on **Plate 4** – **Site Plan** and in **Appendix B** - **Engineered Site Plan**. Discussions on the irrigation wells construction and yields are presented in Section 3.4 and 3.5 of this Report. The approximate location of the proposed outdoor cultivation area, wells, and other site features are also shown on **Plate 4** – **Site Plan** and in **Appendix B** - **Engineered Site Plan**.

Irrigation water will be pumped from the irrigation well to water storage tanks. From the tanks the water will be distributed to the cultivation areas. To conserve water resources the proposed cultivation operation will utilize drip irrigation systems. The project plans do not involve any water diversions, or imported water so all project water will be derived from the project irrigation wells. Details on the cultivation projects water usage, including breakdowns of average and peak monthly usage, are presented in **Table 1 – Total Project and Site Water Usage**.

#### 3.1 CULTIVATION WATER USAGE

The applicant plans to cultivate up 8-acres (348,480 ft²) outdoor cultivation areas for 150 days and 0.92 acres (39,936 ft²) mixed light in greenhouses year-round. The applicant has not had any specific experience growing cannabis at this location but is working with experienced cannabis cultivators and is designing the system to use the least amount of water possible.

It is our understanding that a cannabis water usage rate of 2 acre-feet/acre/year for outdoor cultivation and 3 acre-feet/acre/year for indoor or mixed light cultivation is generally consistent with northern California averages. The applicant estimates that this 8-acre cultivation project will require a total of 5,213,616 gallons/year (16 acre-ft/year) and the mixed light green house area will require 896,225 gallons (2.75 acre-feet/year) as detailed on **Table 1 – Total Project and Site Water Usage.** 

Therefore, a total of 18.75 acre-feet or 2.1-acre feet/acre/year of groundwater will be required for project irrigation.

#### 3.2 RESIDENTIAL WATER USE

There are no residential buildings onsite and there are no immediate plans for residential development onsite. Therefore, residential water use was not a factor in this assessment.

#### 3.3 EMPLOYEE WATER USAGE

We understand that the project will require two full-time farm mangers, as well as, and multiple part-time employees. Therefore, for the purpose of this Assessment we estimate that the project will require an average of eight (8) seasonal employees throughout the growing season (150 days) and two year-round employees. Potable water for farm workers will come from the two irrigation wells. Using the Napa County Water Availability Guidance Document<sup>3</sup> estimate of 15 gallons of water utilized per day per cultivation worker on site. As shown on **Table 1 – Total Project and Site Water Usage** Employee Water Usage was calculated as follows:

Part time Workers= 8 (seasonal employees) x 15 gallons/day (daily water use) x 150 days/year = 18,000 gallons/year

Full time workers = 2 (year-round employees) x 15 gallons/day x 365 days/year = 10,950 Total worker water use = 18,000 (seasonal employees) +10,950 year-round employees =

28,950 gallons /year = 0.09 acre-feet/year = Employee Groundwater Use

#### 3.4 TOTAL PROJECT WATER USAGE

The annual project water use estimate is:

5,213,616 (outdoor cultivation + 896,225 gallons (mixed light cultivation) + 28,950 gallons (employee) = **6,138,791 gallons or 18.84 acre-feet/year = Total Site Water Usage** 

TABLE 1 – TOTAL PROJECT AND SITE WATER USAGE

Source	Jan	Feb	Mar	Apr	May	June	July	Aug	Sept	Oct	Nov	Dec	Total	Total
Source	Gallons													
Outdoor Cultivation	0	0	0	0	0	800,000	1,150,000	1,250,000	1,250,000	763,616	0	0	5,213,616	15.99
Mixed Light Cultivation	80,000	80,000	80,000	80,000	80,000	80,000	80,000	80,000	80,000	80,000	50,000	46,225	896,225	2.75
Full Time Employees	912	913	912	913	912	913	912	913	912	913	912	913	10,950	0.03
Part Time	0	0	0	0	0	3,600	3,600	3,600	3,600	3,600	0	0	18,000	0.06
TOTAL USAGE	80,912	80,913	80,912	80,913	80,912	884,513	1,234,512	1,334,513	1,334,512	848,129	50,912	47,138	6,138,791	18.83

<sup>&</sup>lt;sup>3</sup> Water Availability Analysis (WAA) Guidance Document, Napa County, Adopted May 12, 2015.

Based on these estimates for onsite water use it appears that average water use for the total project would be **16,819 gallons/day** (6,138,791 gallons/365 days). During the peak cultivation season (July- September), the applicant will use a daily maximum of approximately **42,430 gallons/day**.

#### 3.5 SITE WELL INFORMATION

There are four water supply wells located on the site, two of the wells (A and B) will be used for cultivation and one well (# 0952152) supplies a hunting cabin on parcel APN 010-055-33. The Well Completion Reports for all site wells are presented in **Appendix C** and well testing for the two proposed project irrigation wells are presented in **Appendix D**. Well locations are shown on the **Site Plan, Plate 4** and on the **Engineered Site Plans, Appendix B**.

The two proposed irrigation wells (A and B) were constructed and tested in 2021 and have yields between 68 and 70 gallons per minute (gpm). The Well Completion Report for well A shows that the well was completed to a total depth of 395 feet below grade (bg), has a 5-inch diameter PVC well casing, and is screened with 100-feet of slotted sections. Well B was completed to a total depth of 300 feet bg), has a 4.5-inch diameter PVC well casing, and is screened with 129 feet of slotted sections. Both irrigation wells appear to be screened into permeable members of the Franciscan complex rocks (Sandstone, greenstone and gravels).

On November 18, and 19, 2021, well yield tests were conducted on both irrigation wells A and B by JAK Drilling & Pump, **Appendix D** – **Well Yield Test.** Each well was pumped for 6 hours and recovery was monitored for 30 minutes after the pumping stopped. Well A was pumped at 70 gpm with a stabilized drawdown of 22 feet. Based on these measurements, the specific capacity for the irrigation Well A was calculated to be 3.18 gpm/foot of drawdown (70 gpm/22 feet). Recharge was measured after the pumping ceased and within 30 minutes the water level in the well had recovered 100%. Well B was pumped at 68 gpm with a stabilized drawdown of 62 feet. Based on these measurements, the specific capacity for the irrigation Well B was calculated to be 1.10 gpm/foot of drawdown (68 gpm/60 feet). Recharge was measured after the pumping ceased and within 30 minutes the water level in the well had recovered 100%. Of the two wells, Well A produced a higher flow rate with less drawdown resulting in a specific capacity value nearly three times higher. It is likely that well A can sustainably produce greater the than 70 gpm recorded if a larger pump were utilized.

Results of the well yield test indicate the irrigation wells have a combined capability of producing 138 gpm for at least 6-hours without overdrawing the aquifer. The average daily water demand at the site over the cultivation season is expected to be **25,411 gallons/day** gallons/day. Pumping at 138 gpm this would require 184 minutes (3 hours and 4 minutes) of pumping a day to produce that volume of water. The peak daily water demand of **65,256** gallons/day would require approximately 473 minutes (7 hours 52 minutes) of pumping.

**TABLE 2 – Site Well Details** 

APN/ Well Number	Well install/ Test Year	Distance to Site Well* (Feet)	Surface Elevation (Feet)	Total Well Depth (Feet)	Screen Interval (Feet)	Total Screen Thickness (Feet)	Well Yield* (GPM)	Draw- down* (Feet)	Specific Capacity	Aquifer material	Watershed
Well A/ 010- 055-29	2021	Site well	1620	395	160-200 340-400	100	70	22	3.18	Sandstone and greenstone	Burns Valley- Frontal Clearlake
Well B/ 010- 055-28	2021	Site well	1631	300	180-300	120	68	62	1.10	Cemented Franciscan Gravel	Burns Valley- Frontal Clearlake
0952152/ 010- 055-33	2015	2,901**	1847	395	240-395	155	7	NA	NA	Gravelly clay	Grizzly Creek- North Fork Cache Creek
0952153/ 010- 055-28	2015	3,901**	1746	387	220-387	167	5	NA	NA	Gravelly clay	Grizzly Creek- North Fork Cache Creek
Ave	rage Well	TD = 369 f	t bgs	Aver	age Screen '	Screen Thickness = 136 ft Average Specific Capacity					y = 2.14

NA - Not available \* From JAK testing in November 2021 \*\* distance to Well A

#### 4.0 WATER BALANCE INFORMATION

#### 4.1 PRECIPITATION

Precipitation, primarily as rainfall is the major source of inflow aquifers in this area. Though there are no climate stations on site or in the immediate vicinity, we estimate that the seasonal precipitation for the site is 31.42-inches/year<sup>4</sup>. Based on this precipitation it can be reasonably expected that approximately 2.62 acre-feet of rain falls on every acre of the site annually, or 523.61 acre-feet over the three cultivation parcels which total to 199.85 acres.

#### 4.2 GROUNDWATER STORAGE

From data obtained from the 2006 Lake County Water Inventory and Analysis<sup>5</sup> specific yields for aquifers proximate to the site range between 3% - 8%. For the purpose of this assessment, we have assumed a conservative specific yield value of 4%. As discussed in Section 3.6 of this Report, well log information indicated that the well screen interval of the irrigation wells were 100 feet (Well A) and 120 feet (Well B). Therefore, if we assume that the aquifer thickness is consistent with the average well screen interval (110 ft) we can use this value along with regional specific yields to estimate an aquifer storage at the site.

Average Aquifer Thickness (110 feet) x Specific Yield (0.04) x Project Parcels (199.85-acres) = Estimated Aquifer Storage = 879.34 acre-feet

#### 4.3 GROUNDWATER RECHARGE

Groundwater recharge is the replenishment of an aquifer with water from the land surface. It is usually expressed as an average rate of inches of water per year, similar to precipitation. Thus, the volume of recharge is the rate times the land area under consideration times the time period, and is usually expressed as acre-feet per year. In addition to precipitation, other sources of recharge to an aquifer are stream and lake or pond seepage, irrigation return flow (both from canals and fields) inter-aquifer flows, and urban recharge (from water mains, septic tanks, sewers, drainage ditches).

The depth to the static water levels were much higher than the first encountered water levels when drilling which suggests that the aquifer at the site is semi-confined to confined. Drainage features that intersect and border the site have likely eroded through some of the overlying layers and are contributing to the recharge of the site's aquifer through the stream bottom. However, it is also likely that a portion of the rain water falling directly on the site infiltrates the ground surface and migrates downward through the soil matrix until it recharges the aquifer. In addition, flow in the intermittent creek from the north may contribute to recharge of the aquifer near the cultivation areas.

<sup>&</sup>lt;sup>4</sup> http://rainharvestcalculator.com/Rainfall/CA/Clearlake/95422 based on 30-year average (1981-2010)

<sup>&</sup>lt;sup>5</sup> Lake County Water Inventory and Analysis, California Department of Water Resources, March 2006.

To estimate the groundwater recharge at the site we first assumed that the recharge to the aquifer is primarily through rainfall and that all rainfall accumulated within the three cultivation parcels (199.85-acre) drains to onsite and near-site creeks. Therefore, the annual precipitation available for recharge onsite can be estimated using the following data and equation.

199.85acres x 2.62 feet (annual precipitation on the Cultivation Parcel) = Estimated Annual Precipitation Onsite = 523.61 acre-feet

However, this estimate does not account for surface run-off, stream underflow, and evapotranspiration that occurs in all watersheds. According to the USGS, the long-term average precipitation that recharges groundwater in these northern California regions is approximately 15 percent but can be as low as 1.67%. Since this site has relatively mixed topography with both upland and low-lying areas, we estimate that the long-term average precipitation that recharges groundwater within the entire site is slightly below the regional average of 15%. With this data and the precipitation data presented above, we can re-calculate the groundwater recharge within the cultivation parcel using the following equation.

523.61 acre-feet (annual precipitation onsite) x 0.15 (long term average for recharge) = **Estimated Average Groundwater Recharge = 78.54 acre-feet/year** 

The total site water usage is estimated to be **18.83 acre-feet/year** and the groundwater recharge is estimated to be **78.54 acre-feet/year**. Therefore, it appears that the project will have enough water to meet the demands without causing overdraft conditions.

#### 4.3.1 DROUGHT CONDITIONS

The recharge assessment was based on a recent 30-year average for rainfall in the region (1981-2010). However, this average, did not account for severe drought conditions as we have seen over the past 2 years (2019-2020). If we were to assume drought conditions by using a value of 50% of the 30-year average rainfall used above, and assume that the groundwater recharge rate will be reduced to 12%, we can estimate the potential drought condition or low-end value for annual aquifer recharge as follows.

523.61 acre-feet (average precipitation onsite) x 0.5 (drought factor) x 0.12 (conservative long-term average for recharge) =

Estimated Severe Drought Value for Groundwater Recharge = 31.42 acre-feet/year

#### 5.0 PUMPING INFLUENCE TO SURROUNDING PROPERTIES

To evaluate potential well pumping impacts to surface water bodies or wells on other properties, the potential lateral extent of pumping from the planned project well was estimated. Using general relationships discussed in Driscoll (1986)<sup>6</sup>, we estimated the lateral pumping influence using information from the November 2021, well yield test performed by JAK (**Appendix D** – **Well Yield Tests**). An approximate relationship between specific capacity calculated from the well yield testing, and aquifer transmissivity was used to obtain aquifer characteristics and estimate a potential radius of pumping influence. Transmissivity was estimated for an unconfined aquifer and confined aquifer, using the relationship of Specific Capacity (yield/drawdown) x the coefficient of 1,500 (unconfined) and 2,000 (confined). To develop the slope of the drawdown curve from the pumping well, the value of  $\Delta s$  (drawdown over one log graph cycle) was calculated for a distance-drawdown relationship, where  $T = 528Q/\Delta s$  (Driscoll,1986, Equation 9.11), where Q = flow rate in gallons per minute (gpm). The analysis is shown on the attached semi-log plots for the two site irrigation wells (**Appendix E** – **Radius of Pumping Influence**).

The specific capacity for the irrigation well A was calculated to be 3.18 gpm/foot drawdown (70 gpm/ 22 feet drawdown) and for well B, 1.10 gpm/foot drawdown (68 gpm/ 62 feet drawdown). Using this data and applying it to the site, we calculated a zone of pumping influence for confined aquifer conditions. Based on the calculations, it is estimated that the maximum radius of influence for both irrigation wells A and B is 1,200 feet (**Appendix E – Radius of Pumping Influence**).

Wells A and B are located about 1,200 feet apart which indicates they may have some influence on each other if pumped simultaneously, however drawdowns were relatively low so impacts may not be easily measured. The other site wells are not expected to be impacted by pumping of the irrigation wells because they are located outside the calculated radius of pumping influence and in an adjacent watershed (**Site Plan, Plate 4**).

There is a Class II intermittent watercourse that runs adjacent to Ogulin Canyon Road that is approximately 300 feet east of irrigation well A and approximately 700 feet east of irrigation well B. Based on the well completion report data, the wells are drawing from a confined or semi-confined aquifer (> 100 feet depth) and therefore do not appear to have any direct connection with this intermittent watercourse. Therefore, pumping from the irrigation wells is not expected to impact this creek.

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<sup>&</sup>lt;sup>6</sup> Groundwater and Wells, Second Edition, Fletcher G. Discoll, 1986, published by Johnson Division, St. Paul Minnesota, 1089p.

#### 6.0 WATER QUALITY

During well yield testing, water samples were collected from the domestic well by JAK Drilling and Pump and tested for, Hardness, Iron (ferrous), pH and Total dissolved solids (TDS). The results of the water testing are presented in **Table 3 – Water Quality Data** and the laboratory analytical reports are included with the well yield testing forms in **APPENDIX D – Well Yield Tests**.

**TABLE 3 – WATER QUALITY DATA** 

Location	pН	Iron (mg/L)	TDS (mg/L)	Hardness (g/g)
Well A	7.5	1.5	426	9
Well B	7.5	3.8	425	9
California Secondary Drinking Water Standards	NA	0.3	500	10
NA – Not Applicable	·		·	-

#### 7.0 CONCLUSIONS

The site is located within two watersheds however the cultivation areas are in the Burns Valley - Frontal Clearlake Sub-watershed and in the Burns Valley Aquifer area. The two project irrigation wells (A and B) are drawing groundwater from a semi-confined to confined aquifer consisting primarily of sedimentary rocks (Gravels, sandstone and greenstone). Recharge to the groundwater likely occurs primarily from direct precipitation and percolation as well as from stream flow from onsite creeks. The estimated groundwater usage for the entire project including employees is approximately 18.83 acre-feet/year. Average annual recharge available to the site aquifer is estimated at 78.54 acre-feet/year. Based on well yield test data collected at the site, it appears that the aquifer storage and recharge area are sufficient to provide for sustainable annual water use at the site and within the area.

#### In summary:

Estimated Cultivation Irrigation Water Use – 18.74 acre-feet/year Site Worker Water Use - 0.09 acre-feet/year Total Estimated Site Water Use – 28.45 acre-feet/year Estimated Annual Recharge – 78.28 acre-feet/year Estimated Recharge including Severe Drought – 31.42 acre-feet/year Irrigation Wells Combined Sustainable Pumping Rate – 138 gpm Average Daily Water Demand for Cannabis – 16,819 gallons/day Peak Daily Water Demand for Cannabis – 42,430 gallons/day

- The quantity of groundwater to be used for the project compared to the average quantity of available groundwater indicates that pumping for the proposed project is unlikely to result in significant declines in groundwater elevations or depletion of groundwater resources over time.
- The horizontal and vertical separations between the project wells and the nearest neighboring properties should not result in significant well interference. Potential impacts to nearby intermittent or ephemeral watercourses are also not considered a concern to this assessment.
- Only a limited water quality assessment has been performed and therefore we recommend that the
  project water supplies be tested for bacteria and common contaminants such as arsenic and nitrates
  before being utilized as a potable water source.

#### 8.0 LIMITATIONS

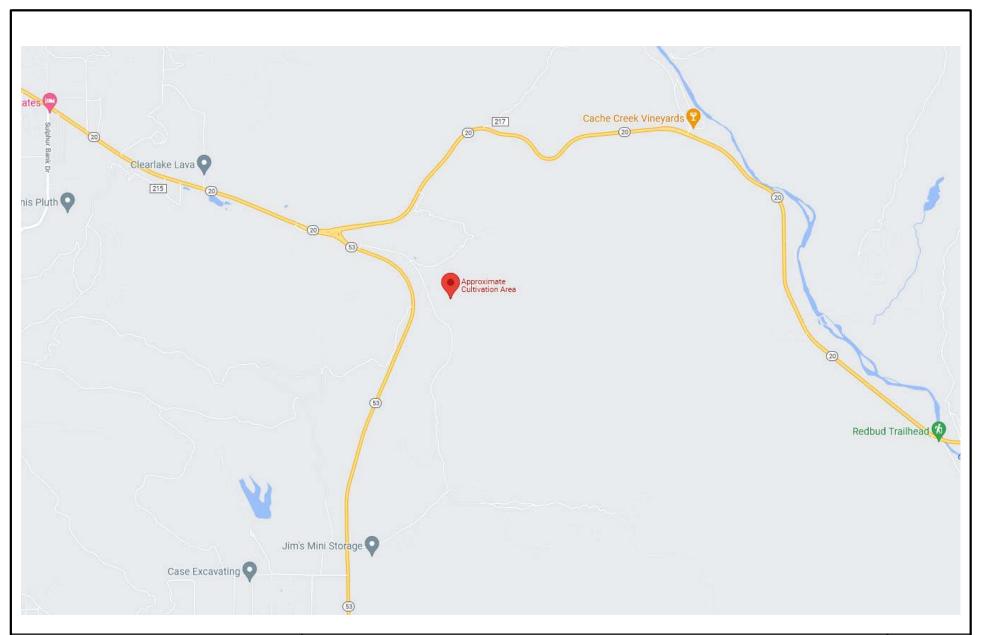
HES is not responsible for the independent conclusions, opinions or recommendations made by others based on the records review, site inspection, field exploration, laboratory test data and interpretations presented in this report.

Groundwater systems of Lake County are typically complex, and available data rarely allows for more than general assessment of groundwater conditions and delineation of aquifers. Hydrogeologic interpretations are based on the drillers' reports made available to us through the California Department of Water Resources, available geologic maps and hydrogeologic studies and professional judgment. This analysis is based on limited available data and relies significantly on interpretation of data from disparate sources of disparate quality.

It should be noted that hydro-geological assessments are inherently limited in the sense that conclusions are drawn and recommendations developed from information obtained from limited research and site evaluation. Additionally, the passage of time may result in a change in the environmental characteristics at this site and surrounding properties. This report does not warrant against future operations or conditions, nor does this warrant operations or conditions present of a type or at a location not investigated.

This study is not intended to assess if any soil contamination, waste emplacement, or groundwater contamination exists by subsurface sampling through the completion of soil borings and the installation of monitoring wells. The scope of work, determined by the client, did not include these activities.

This Report is for the exclusive use of Mr. Simon Whetzel, his affiliates, designates and assignees and no other party shall have any right to rely on any service provided by Hurvitz Environmental Services without prior written consent.



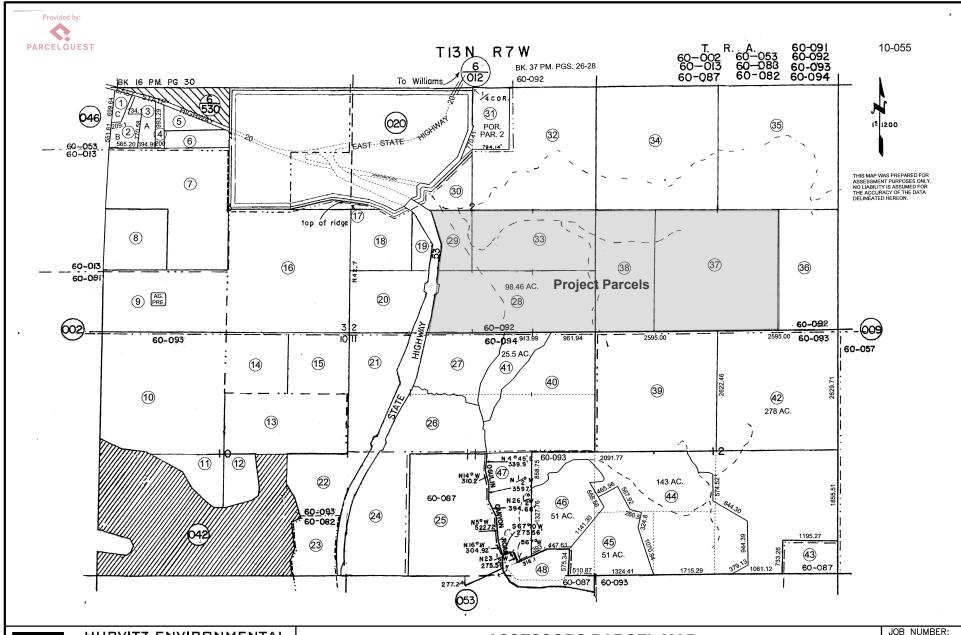


105 MORRIS ST, STE 188
SEBASTOPOL, CA 95472
PH: 707.824.1690
FX: 707.824.2675
HURVITZ.ENVIRONMENTAL@GMAIL.COM
CA PG# 7573

#### **SITE MAP**

010-055-28, 010-055-29, 010-055-33, 010-055-37 and 010-055-38 660 Junction Plaza Clearlake, CA 95423 JOB NUMBER: 5185.01

DATE: 01/28/2022





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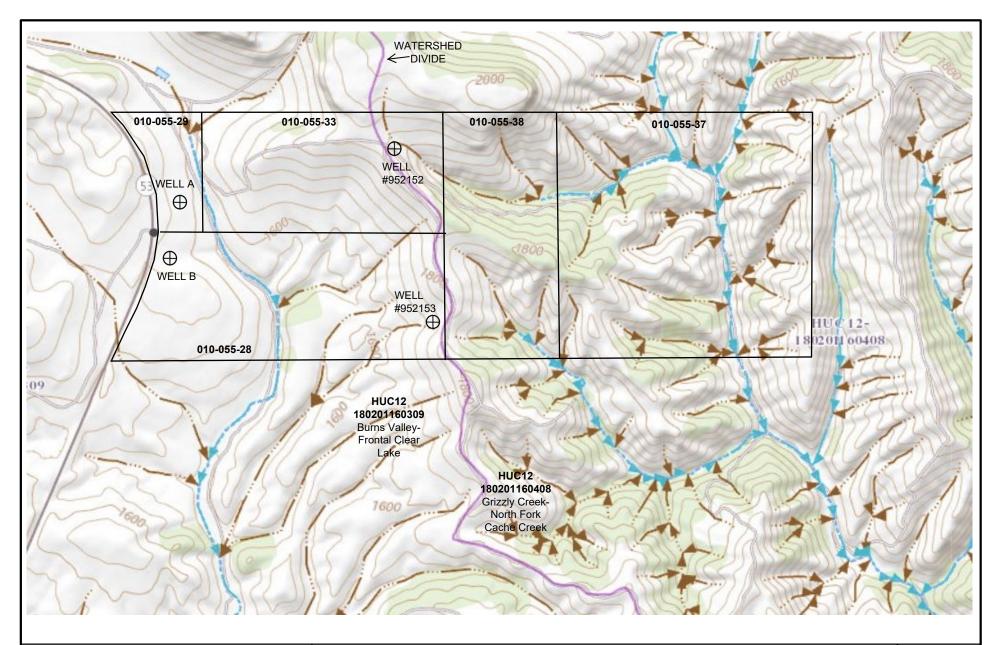
#### **ASSESSORS PARCEL MAP**

010-055-28, 010-055-29, 010-055-33, 010-055-37 and 010-055-38 660 Junction Plaza Clearlake, CA 95423 JOB NUMBER: 5185.01

DATE: 01/28/2022

PLATE:

**2** 



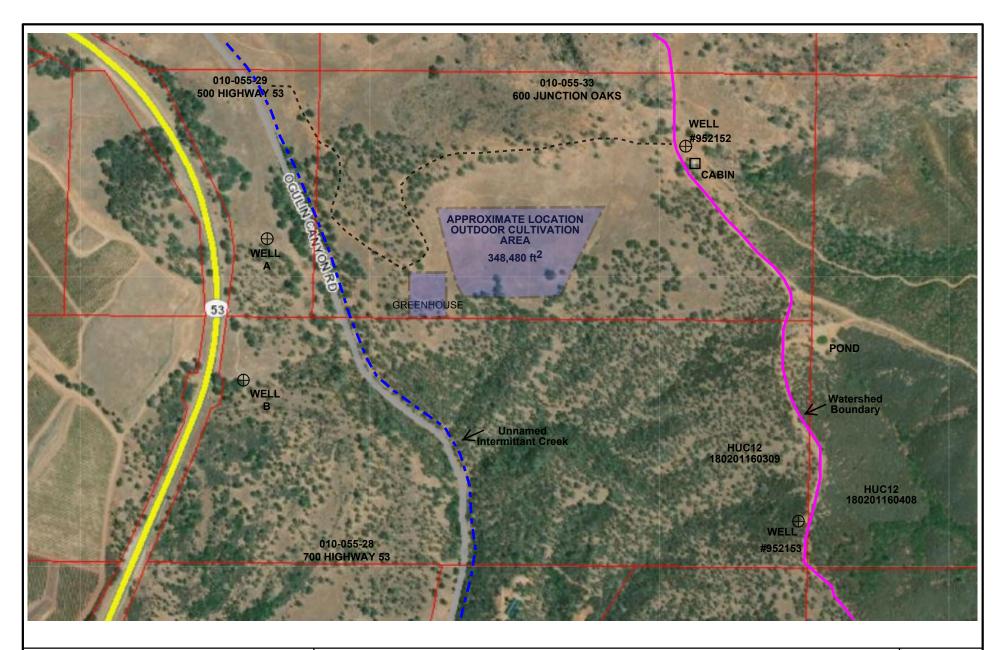


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#### **USGS TOPOGRAPHIC MAP**

010-055-28, 010-055-29, 010-055-33, 010-055-37 and 010-055-38 660 Junction Plaza Clearlake, CA 95423 JOB NUMBER: 5185.01

DATE: 01/28/2022



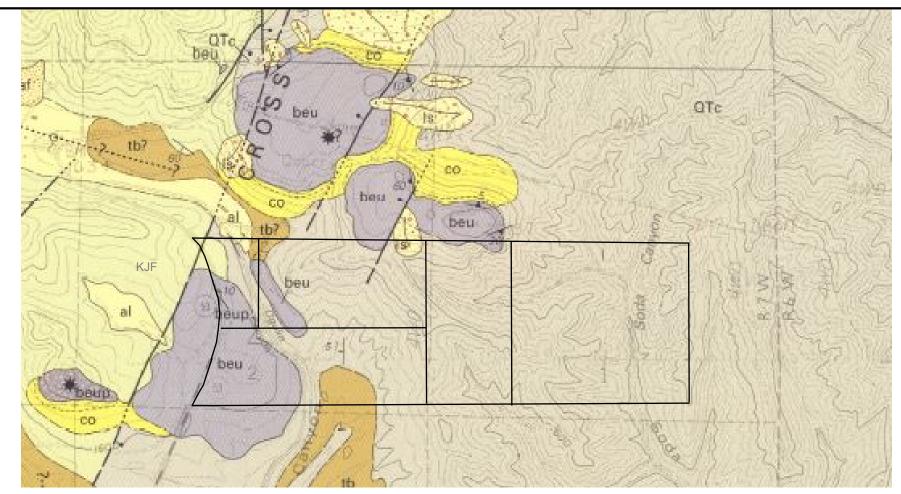


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#### **SITE PLAN**

010-055-28, 010-055-29, 010-055-33, 010-055-37 and 010-055-38 660 Junction Plaza Clearlake, CA 95423 JOB NUMBER: 5185.01

DATE: 01/28/2022



SOURCE: USGS 1995 Geologic Map and Structure section of the Clear Lake Volcanics, Northern California, B.C. Hearn, Jr, J.M. Donnelly-Nolan, and F.E. Goff

- Is Holocene, surficial landslide deposits unsorted angular blocks and soil.
- co Holocene surficial colluvium deposits slope deposits of silt sand and coarser angular clast.
- **al** Holocene alluvial deposits flood plain, channel and lake deposits of clay, silt, sand and gravel.
- $\mbox{\bf QTc}$  Pleistocene and Pliocene Siltstone, sandstone, conglomeratic sandstone and tuff.
- **tb** Pleistocene terrace deposits of the Burns Valley sand and gravel predominately composed of chert and greenstone clast from the Cache Formation.
- **beu** Pleistocene and Pliocene undivided early basaltic rocks- olivine basalt and olivine basaltic andesite dominantly in flows, with some pyroclastic deposits an intrusive rocks.

**KJF** Franciscan Assemblage – Upper Cretaceous to Upper Jurassic- Structurally complex basement rocks includes: chert, greenstone, greywacke, shale and metamorphic rocks of blueschist grade.



#### HURVITZ ENVIRONMENTAL

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CA PG# 7573

#### **GEOLOGIC MAP**

010-055-28, 010-055-29, 010-055-33, 010-055-37 and 010-055-38

660 Junction Plaza Clearlake, CA 95423 JOB NUMBER: 5185.01

DATE: 01/28/2022

# APPENDIX A SITE PHOTOGRAPHS

### **CULTIVATION RELATED PHOTOS**



Location Proposed Outdoor Cultivation/Canopy Area (northeast view)



Location Proposed Outdoor Cultivation/Canopy Area (west view)

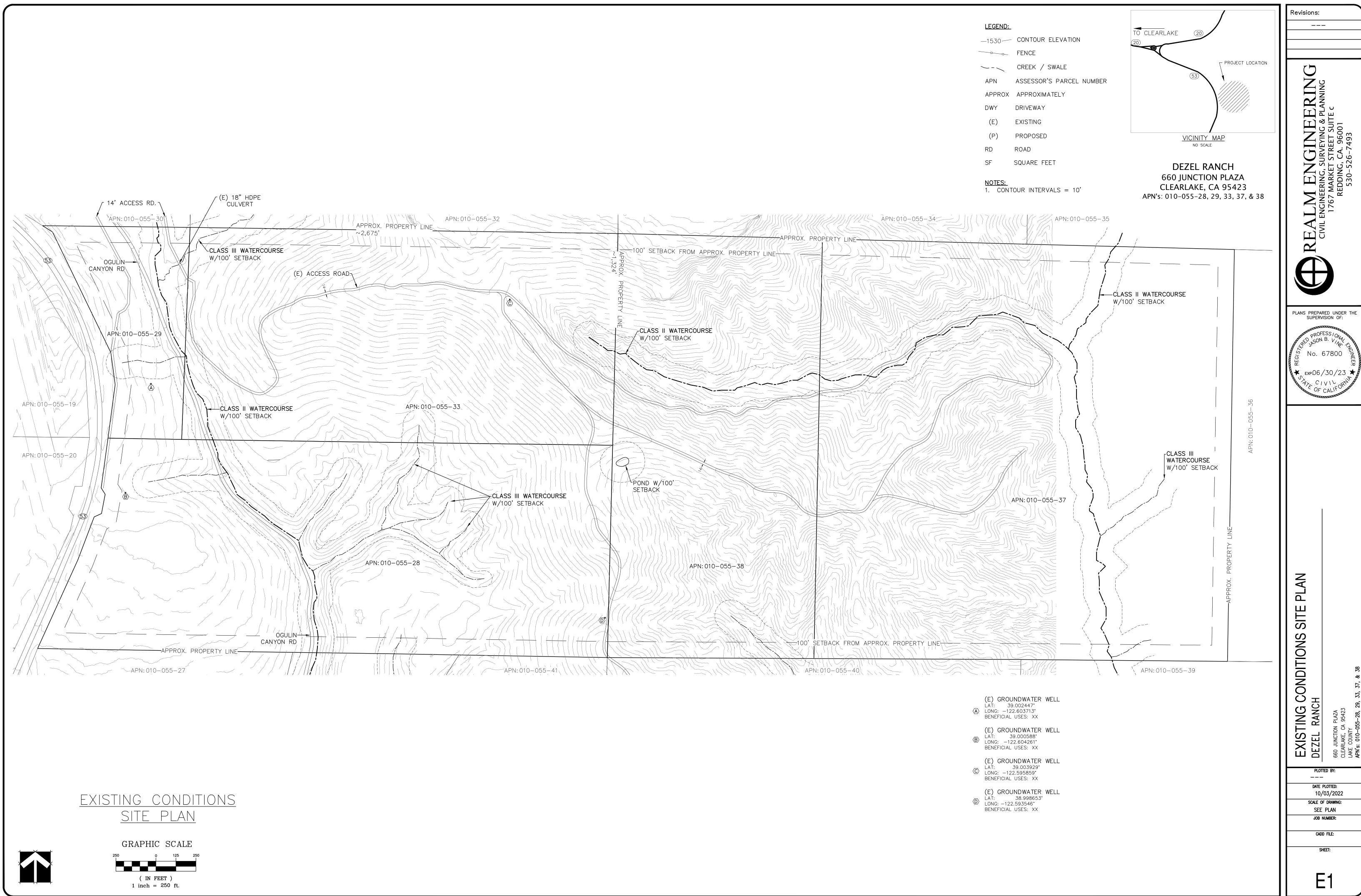


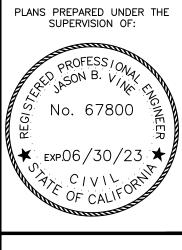
Existing Native Soil Surfaced Access Road of Project Property.

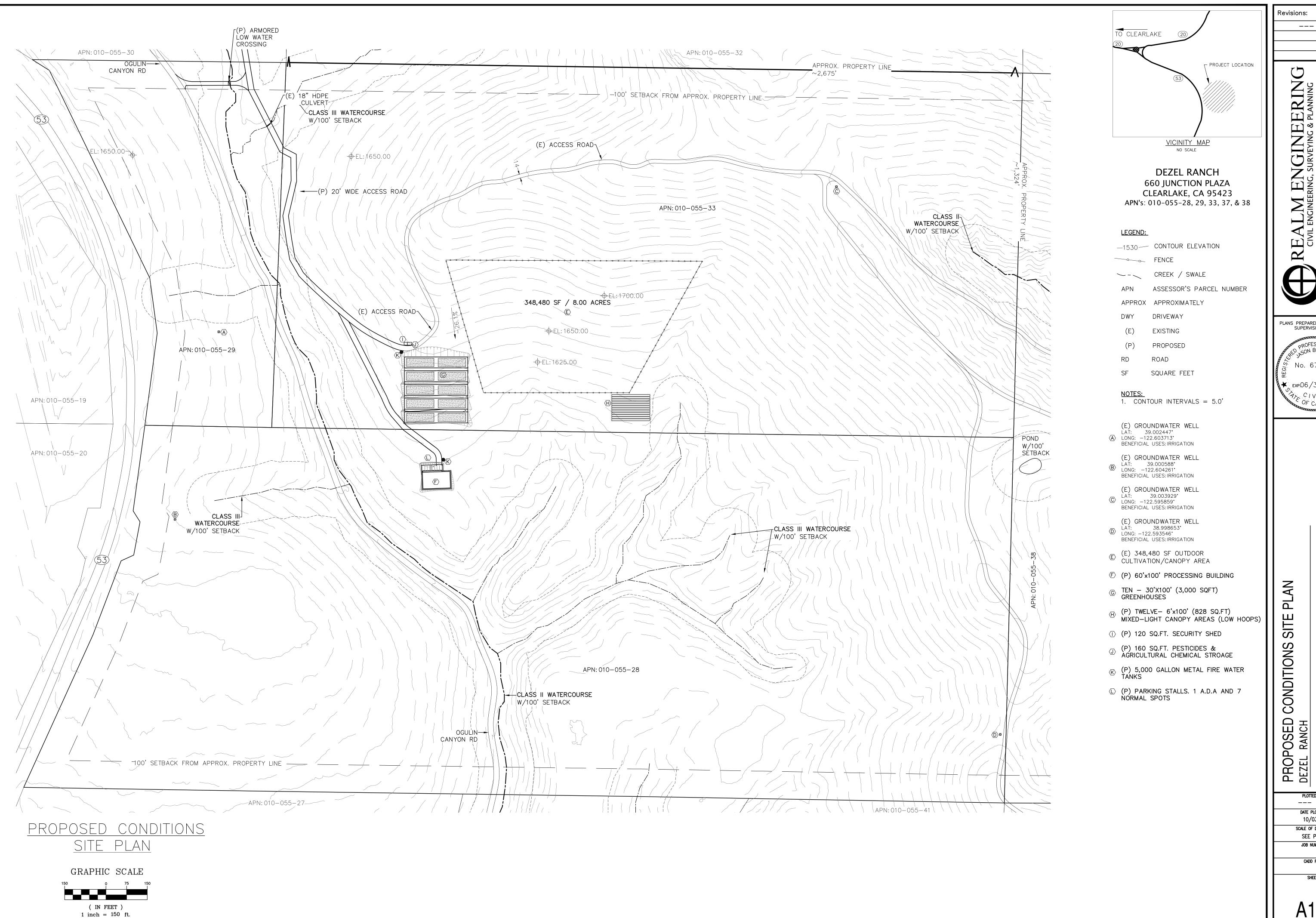


Existing Native Soil Surfaced Access Road of Project Property.

#### APPENDIX B ENGINEERED SITE PLANS







Revisions:

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ENING, SURVEYING & PLANNING MARKET STREET SUITE CA. 96001 530-526-7493

PLANS PREPARED UNDER THE SUPERVISION OF:

A RED ASON B. V. ? No. 67800 **√** EXP.06/30/23 →

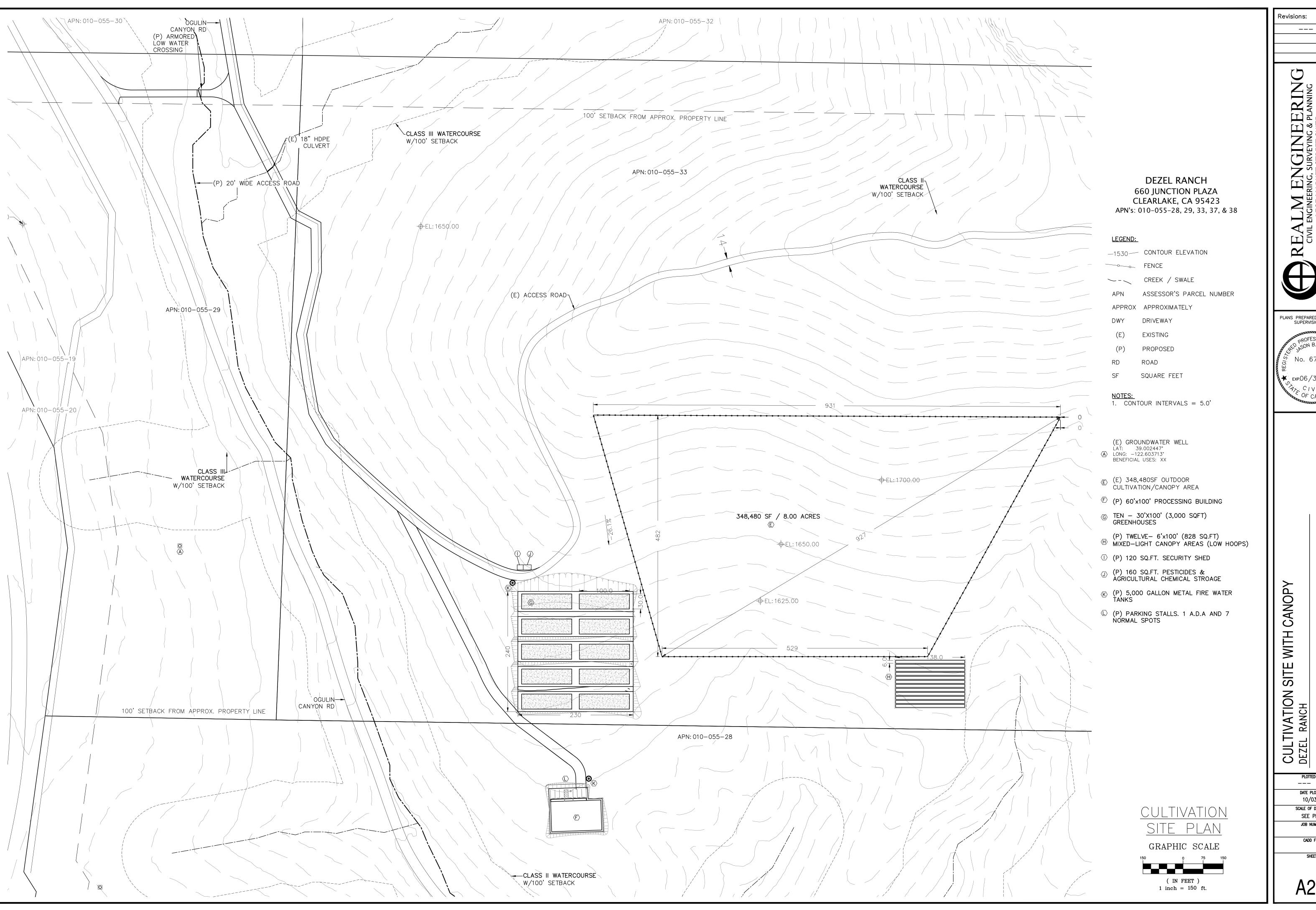
SITE CONDITIONS

PLOTTED BY:

\_\_\_ DATE PLOTTED: 10/03/2022 SCALE OF DRAWING:

SEE PLAN JOB NUMBER: CADD FILE:

A1.2



Revisions:

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ENGLENCE SURVEYING & PLANNING MARKET STREET SUITE C EDDING, CA. 96001 530-526-7493

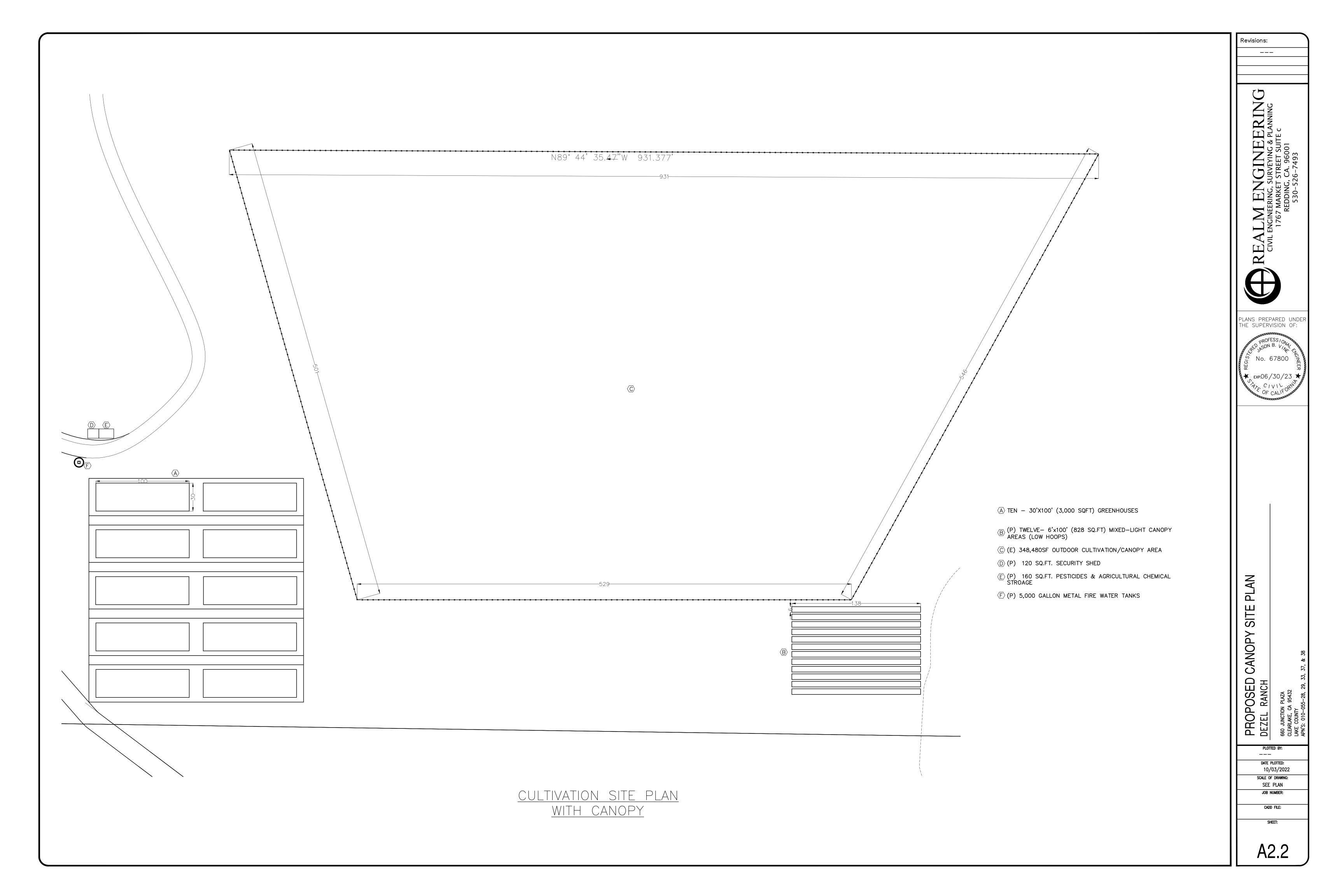
PLANS PREPARED UNDER THE SUPERVISION OF: PROFESS/ No. 67800 EXP.06/30/23

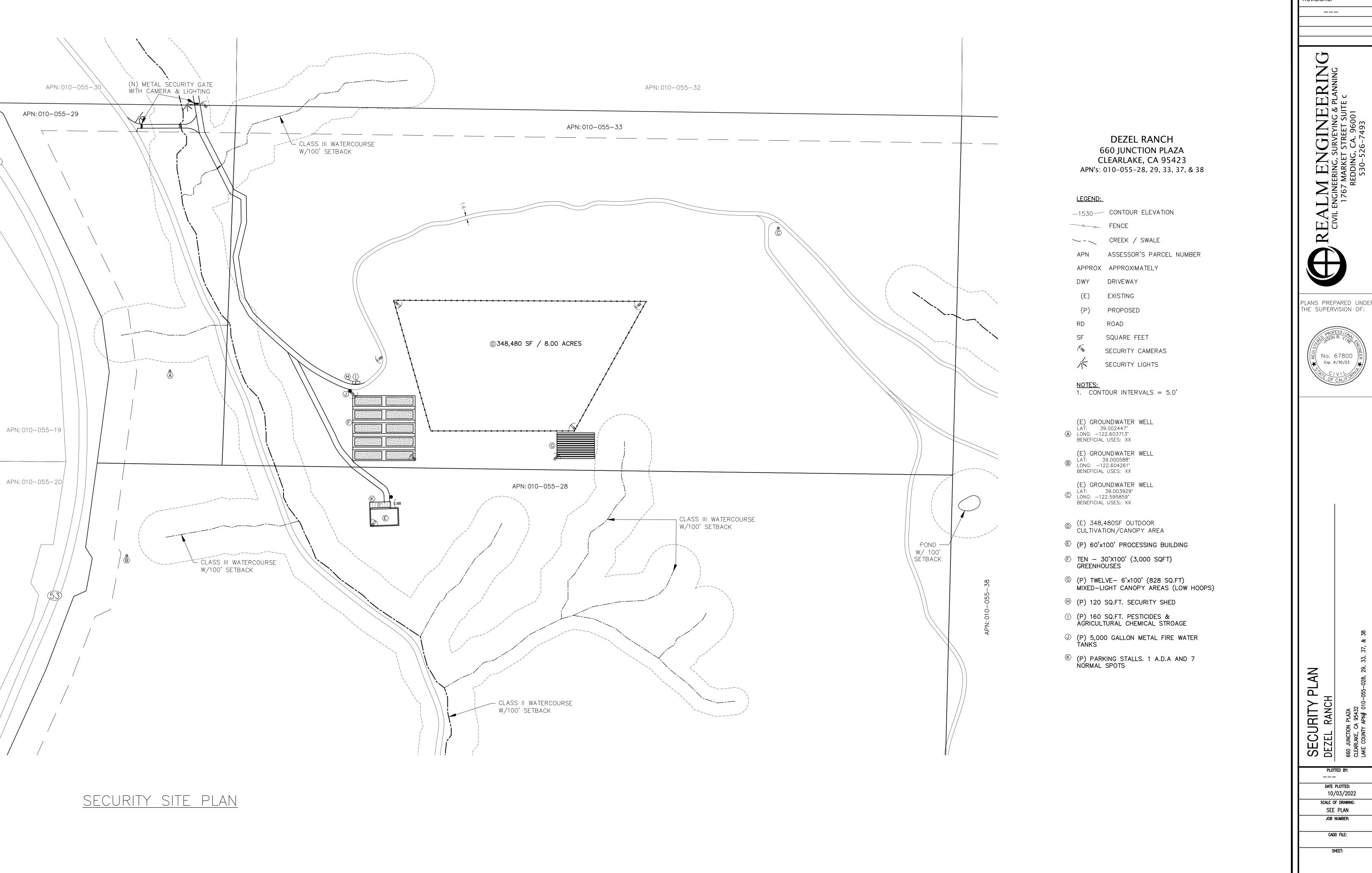
PLOTTED BY: \_\_\_

DATE PLOTTED: 10/03/2022

SCALE OF DRAWING: SEE PLAN JOB NUMBER:

CADD FILE:





Revisions:



PLANS PREPARED UNDER THE SUPERVISION OF:

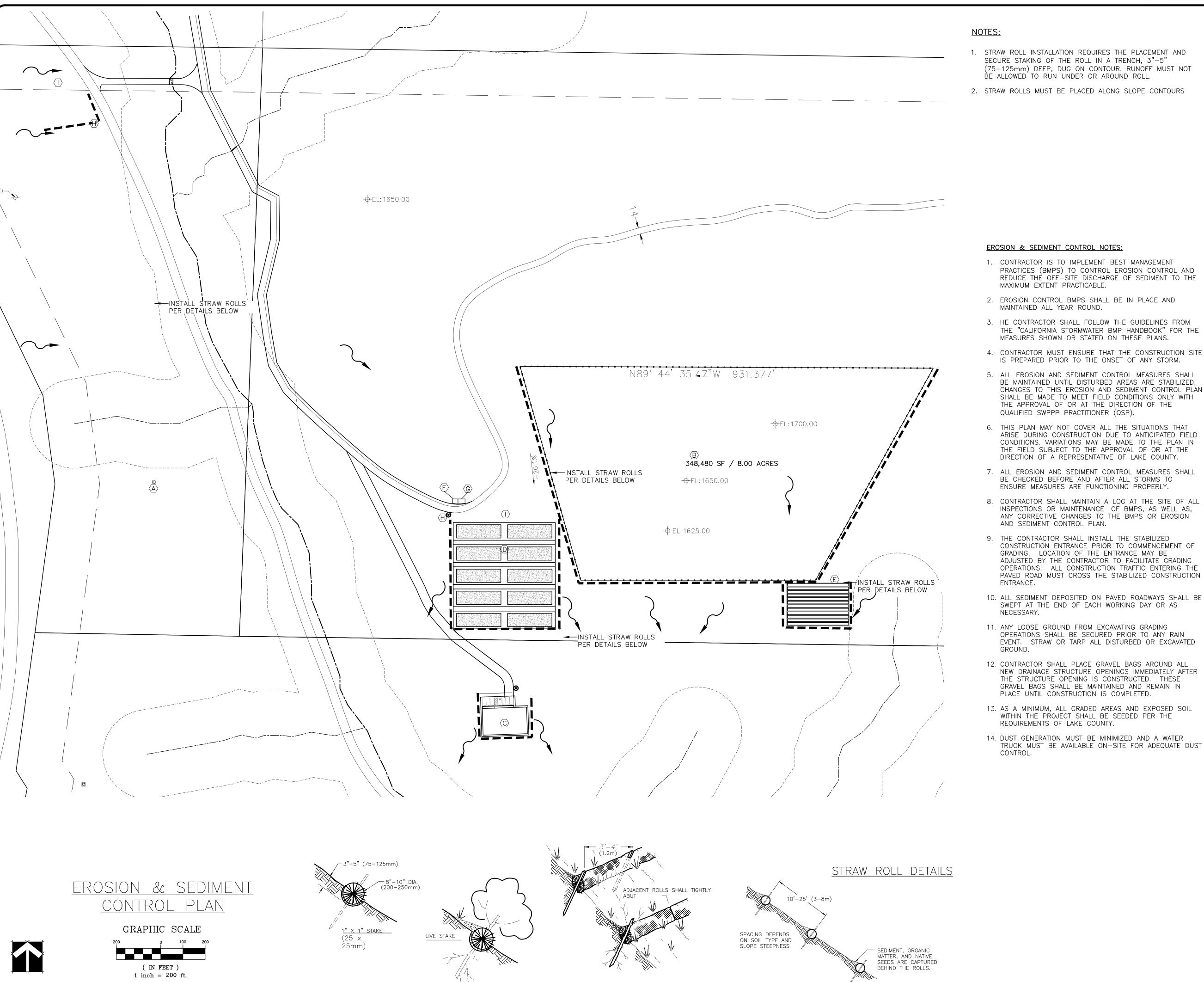


PLOTTED BY:

---DATE PLOTTED: 10/03/2022 SCALE OF DRAWING: SEE PLAN

CADD FILE:

A2.1



- 1. STRAW ROLL INSTALLATION REQUIRES THE PLACEMENT AND SECURE STAKING OF THE ROLL IN A TRENCH, 3"-5" (75-125mm) DEEP, DUG ON CONTOUR. RUNOFF MUST NOT BE ALLOWED TO RUN UNDER OR AROUND ROLL.
- 2. STRAW ROLLS MUST BE PLACED ALONG SLOPE CONTOURS





STRAW ROLLS (ADJUST TO SUIT FIELD CONDITIONS)

Revisions:

\_\_\_



DISCHARGE POINT



CONSTRUCTION MATERIALS STORAGE AREA

## **DEZEL RANCH** 660 JUNCTION PLAZA CLEARLAKE, CA 95423

FENCE

DWY DRIVEWAY EXISTING

PROPOSED

SQUARE FEET

## 1. CONTOUR INTERVAL IS 5.0'

LONG: -122.603713°

(E) 348,480SF OUTDOOR

(P) 60'x100' PROCESSING BUILDING

GREENHOUSES

# APN's: 010-055-28, 29, 33, 37, & 38

#### LEGEND:

—1530 — CONTOUR ELEVATION

ASPHALT

GRAVEL

ASSESSOR'S PARCEL NUMBER

APPROX APPROXIMATELY

(E) GROUNDWATER WELL LAT: 39.002447° BENEFICIAL USES: XX

CULTIVATION/CANOPY AREA

(P) TWELVE 30'x100' GUTTER CONNECTED

 $\bigcirc$  (P) EIGHTEEN -6'x92.5' CUTTER CONNECTED GREEN HOUSES

(F) (P) 120 SQ.FT. SECURITY SHED

(P) 160 SQ.FT. PESTICIDES & AGRICULTURAL CHEMICAL STROAGE

(P) 5,000 GALLON METAL FIRE WATER TANKS

(P) PARKING STALLS. 1 A.D.A AND 7 NORMAL SPOTS

PLANS PREPARED UNDER THE SUPERVISION OF:

No. 67800

EXP.06/30/23

7 REDIASON B. L

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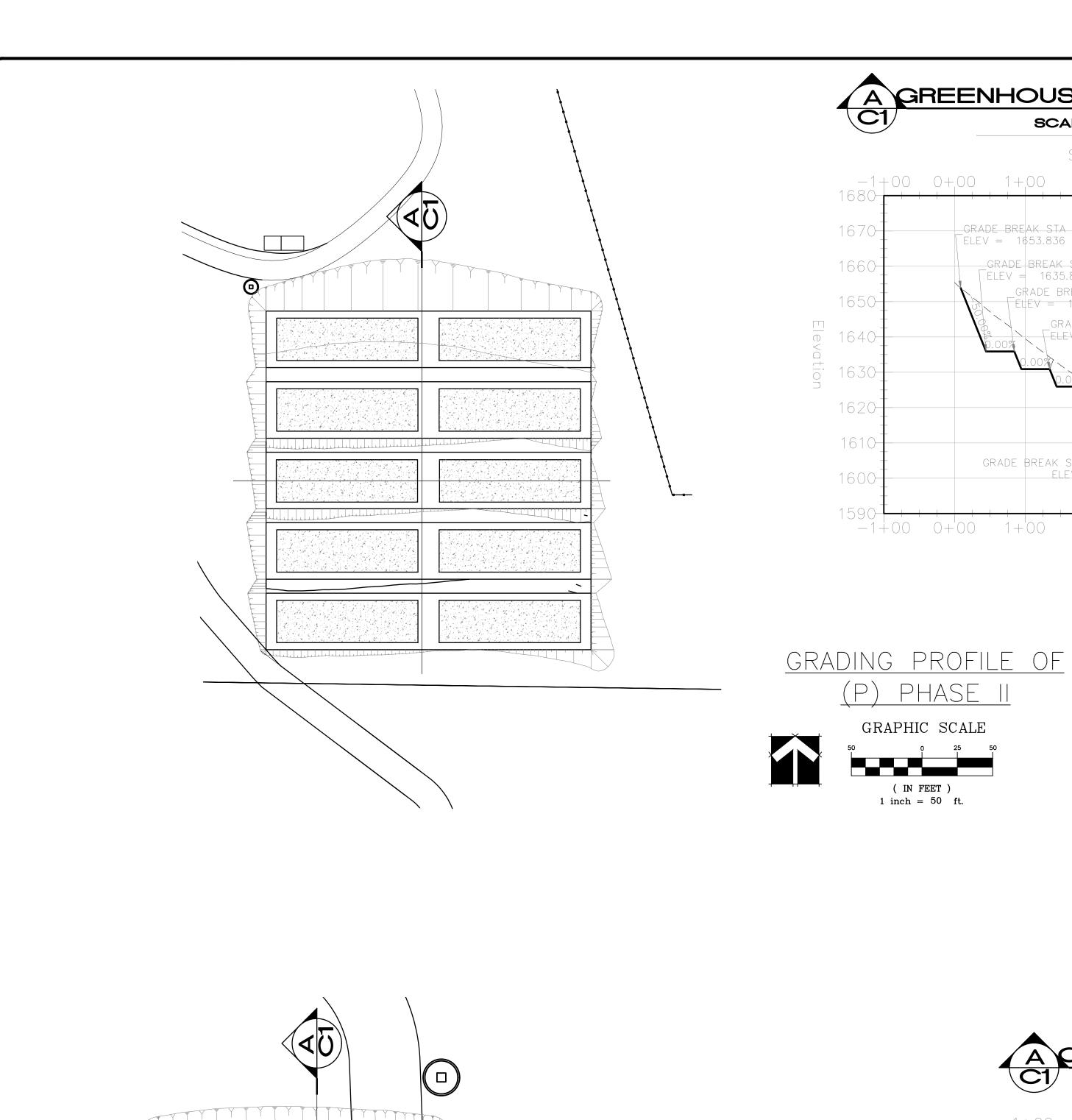
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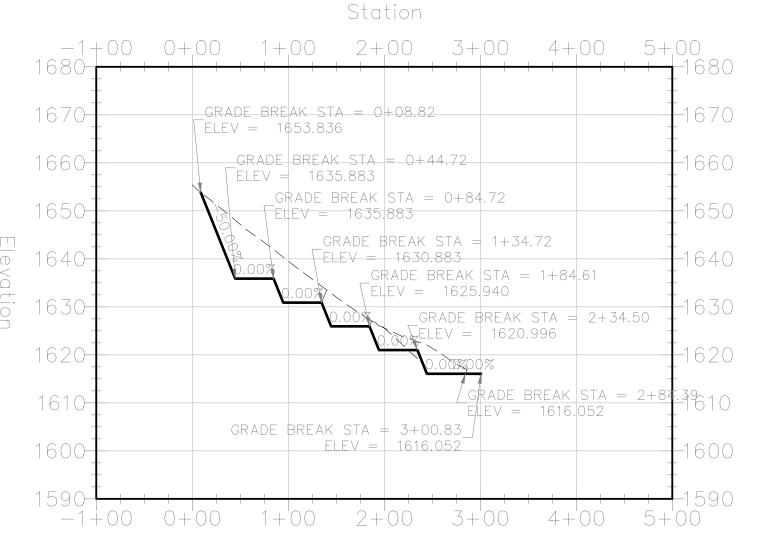
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see plan







<u>(p) phase II</u>

GRAPHIC SCALE

( IN FEET ) 1 inch = 50 ft.

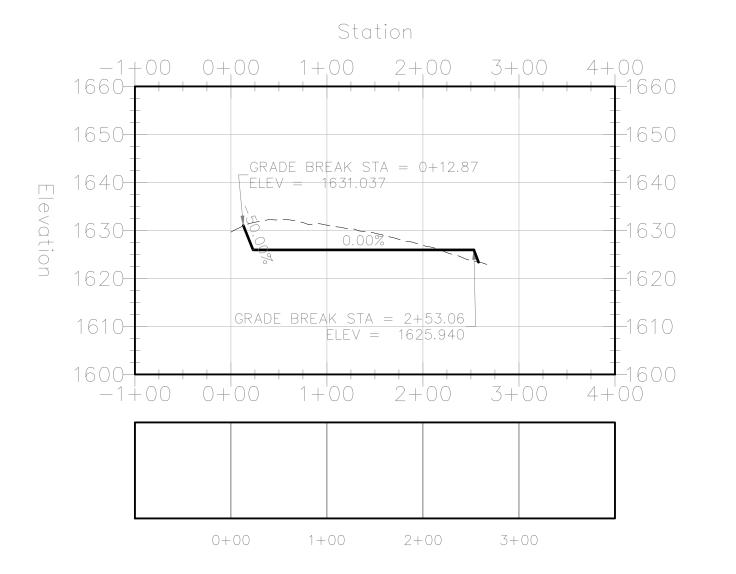
GRADING PROFILE OF

(P) PHASE II

GRAPHIC SCALE

1 inch = 20 ft.

## B GREENHOUSE SECTION B-B' **C1** SCALE: 1" = 100'



B GREENHOUSE SECTION B-B'

0+,00

0+00

STA: 0+13.51

ELEV: 1596.25

SCALE: 1" = 100'

0.00%

ELEV = 1595.314

Station

1+,00

GRADE BREAK STA = 0+11.64

1+'00

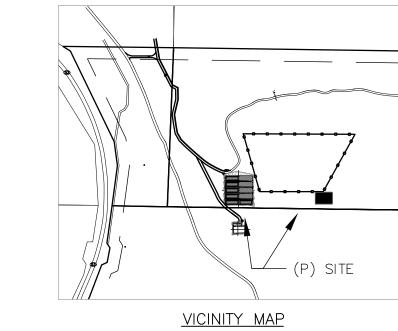
2+,00

2+00

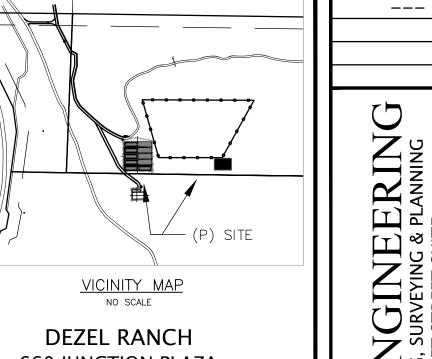
\_GRADE BREAK STA = 1+19.53 1610

GRADE BREAK STA +

 $\sqrt{\text{ELEV}} = 1592.204 - 1600$ 



**DEZEL RANCH** 660 JUNCTION PLAZA CLEARLAKE, CA 95423 APN's: 010-055-28, 29, 33, 37, & 38



PLANS PREPARED UNDER THE SUPERVISION OF:

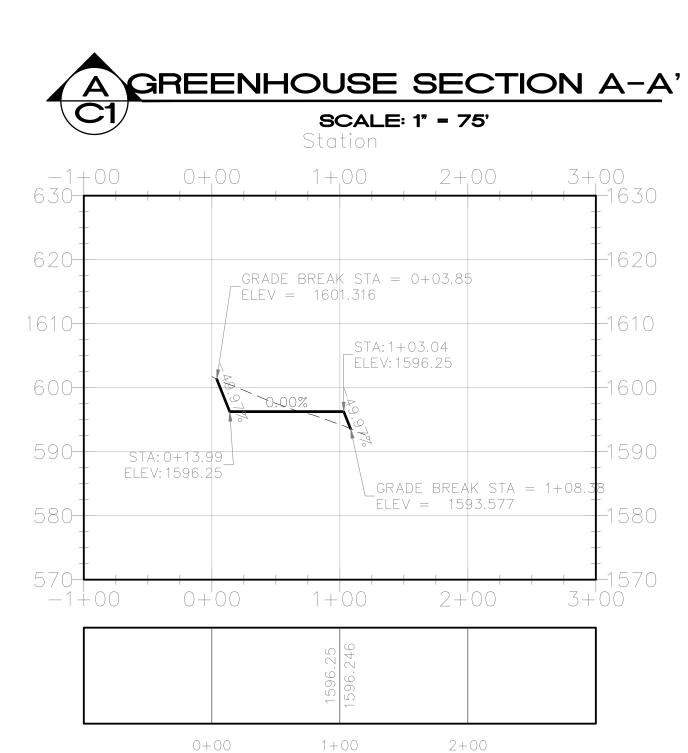
Revisions:

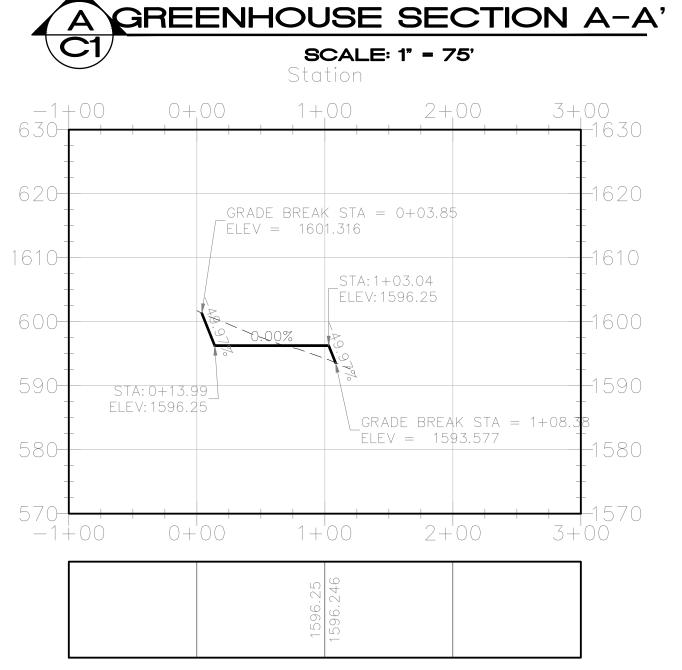
PROFESS, ۷ No. 67800 **√**★ EXP.06/30/2.

SECURITY SITE I

DATE PLOTTED: 10/03/2022 SCALE OF DRAWING: SEE PLAN

CADD FILE:





# APPENDIX C WELL COMPLETION REPORTS

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Total Depth of Completed Well 300 Feet  Casings  Depth from Borehole Diameter Type Material Thickness Diameter Inches)  O 40 9' F480 PUC 14' 18' Plank O 1 2 15 15 10 10 10 10 10 10 10 10 10 10 10 10 10	Total De	pth of Bo	oring		305		Feel				100 L	(Fael	Dale -	Measu	red 4-24-21
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**ORIGINAL** STATE OF CALIFORNIA 3N/ File with DWR NOV 9 2 2015 WELL COMPLETION REPORT Refer to Instruction Pamphlet Page \_\_\_\_ of \_ No. 0952152 Owner's Well No. LATITUDE LONGITUDE Date Work Began Local Permit Agency APN/TRS/OTHER Permit No. WE Permit Date. GEOLOGIC LOG VERTICAL (SPECIFY) ORIENTATION (∠) HORIZONTAL \_\_ ANGLE DRILLING FLUID . METHOD DEPTH FROM SURFACE DESCRIPTION Describe material, grain size, color, etc. City County \_ APN Book 40 Parcel Township. Range \_ Section Long\_ DEG. SEC. DEG. MIN. MIN. SEC. LOCATION SKETCH ACTIVITY (∠) NEW WELL MODIFICATION/REPAIR \_ Deepen \_ Other (Specify) DESTROY (Describe Procedures and Materials Under "GEOLOGIC LOG" USES (∠) WATER SUPPLY
Domestic \_\_\_\_ Public Irrigation \_ \_ Industrial MONITORING TEST WELL CATHODIC PROTECTION HEAT EXCHANGE DIRECT PUSH INJECTION SPARGING SOUTH

Illustrate or Describe Distance of Well from Roads, Buildings, Fences, Rivers, etc. and attach a map. Use additional paper if necessary. PLEASE BE ACCURATE & COMPLETE. REMEDIATION OTHER (SPECIFY) WATER LEVEL & YIELD OF COMPLETED WELL DEPTH TO FIRST WATER (Ft.) BELOW SURFACE DEPTH OF STATIC WATER LEVEL \_ (Ft.) & DATE MEASURED ESTIMATED YIELD (GPM) & TEST TYPE\_ TOTAL DEPTH OF BORING TEST LENGTH \_ \_ (Hrs.) TOTAL DRAWDOWN\_ TOTAL DEPTH OF COMPLETED WELL \* May not be representative of a well's long-term yield. CASING (S) ANNULAR MATERIAL DEPTH FROM SURFACE DEPTH BORE-HOLE FROM SURFACE TYPE (∠) TYPE DIA. CON-DUCTOR INTERNAL GAUGE SLOT SIZE SCREEN FILL PIPE MATERIAL / CE-BEN-FILTER PACK DIAMETER OR WALL IF ANY FILL GRADE MENT TONITE Ft. (TYPE/SIZE) THICKNESS Ft. (Inches) (Inches)  $(\angle)$  $(\preceq)$ X 11 grave ATTACHMENTS (∠) CERTIFICATION STATEMENT I, the undersigned, certify that this report is complete and accurate to the best of my knowledge and belief. Geologic Log Well Construction Diagram Geophysical Log(s) \_ Soil/Water Chemical Analyses ADDRESS \_\_ Other \_ ATTACH ADDITIONAL INFORMATION, IF IT EXISTS.

#### APPENDIX D WELL YIELD TESTS



Date: **11/19/2021** Technician: Jim Jackson Client Name: Simon Whetzel Site Address: APN: 010-055-29 500 State Hwy 53, Clearlake Well Pump Info (size, type, brand, etc.): 10hp, 3-Phase 460V with 55-Series Pump End Power Source (hardwired, generator, solar only, solar with generator back up): Gen Total Depth of Well? **391-feet** Static Water Level? 66-Feet Diameter of Well? 5-inch Casing Type? **PVC** Last time the water was pumped from the well? **New Construction** Was the pumping level measured from ground surface or top of casing? Ground Surface Flow Rate\* Interval Time Pumping Level \*Flow Rate Measured via Bucket or Meter 5 9:30 70 84.0 Meter Start: 5 9:35 70 84.0 5 9:40 70 85.0 425380 Meter Stop: 5 70 85.5 9:45 5 70 86.0 9:50 Gallons Pumped: 25,230 5 9:55 70 86.3 10 70 86.1 10:05 10 10:15 70 86.7 10 70 86.8 10:25 10 10:35 70 87.0 10:45 Field Quality Test Completed (Y/N): Y 10 70 87.1 87.2 10 10:55 70 30 11:25 70 87.3 pH: 7.5 30 11:55 70 87.5 30 12:25 70 88.0 TDS: 426 ppm 30 12:55 70 88.0 30 13:25 70 88.0 Hardness: 9 grains per gallon 30 13:55 70 88.0 30 14:25 70 88.0 Iron: 1.5 ppm 30 14:55 70 88.0 30 15:25 70 88.0 GPS: **39.00253626, -122.60369434 STOP** 

#### 30 Disclaimer:

10

15:35

16:05

RECHARGE

RECHARGE

Observations made of the well(s) are strictly limited to the date and time that the test(s) was conducted and are in no way a guarantee of future conditions, including but not limited to the quantity and/or quality of the water produced by this well.

68

66



Date: <b>11/</b> :	18/2021			Technician:	Jim Jackson						
Client Nan	ne:	Simon Whetzel									
Site Addre	ess:	700 State Hwy	53, Clearlake	APN: <b>010-055-28</b>							
Well Pum	p Info (size, ty	pe, brand, etc.):	10hp, 3-Phase 460\	V with 55-Series Pump E	nd						
				h generator back up): (							
	th of Well? <b>31</b>		, di	Static Water Level? 116-Feet							
Diameter		4.5-inch		Casing Type?	PVC						
		pumped from t	ha wall?	New Construction							
		· · · · · · · · · · · · · · · · · · ·			10. (						
Was the p	umping level	measured from g	ground surface or	top of casing? Ground	1 Surface						
Interval	Time	Flow Rate*	Pumping Level	*Flow Rate Measured	d via Bucket or Meter						
5	8:00	68	156.0	Meter Start:	375670						
5	8:05	68	165.0	1							
5	8:10	68	169.0	Meter Stop:	400150						
5	8:15	68	171.0	1 -							
5	8:20	68	171.2	Gallons Pumped:	24,480						
5	8:25	68	172.0	1	<del></del>						
10	8:35	68	173.0	1							
10	8:45	68	174.0	1							
10	8:55	68	174.5	1							
10	9:05	68	175.0	1							
10	9:15	68	175.5	Field Quality Test Cor	mpleted (Y/N): <b>Y</b>						
10	9:25	68	176.1	]							
30	9:55	68	176.5	pH:	7.5						
30	10:25	68	177.0								
30	10:55	68	177.7	TDS: <b>425</b> pp	om						
30	11:25	68	178.0								
30	11:55	68	178.0	Hardness: 9 grain	ns per gallon						
30	12:25	68	178.0								
30	12:55	68	178.0	Iron: 3.8 pp	o <b>m</b>						
30	13:25	68	178.0								
30	13:55	68	178.0	GPS: <b>39.00252839,</b> -	122.60433554						
		STOP		]							
10	14:05	RECHARGE	125.0	]							
30	14:35	RECHARGE	99.0	]							

#### Disclaimer:

Observations made of the well(s) are strictly limited to the date and time that the test(s) was conducted and are in no way a guarantee of future conditions, including but not limited to the quantity and/or quality of the water produced by this well.

# APPENDIX E RADIUS OF INFLUENCE PLOTS

