ATTACHMENT 1

TRAFFIC IMPACT ANALYSIS

FOR

MIDDLETOWN DOLLAR GENERAL STORE

Lake County, CA

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Middletown Dollar General rpt

TRAFFIC IMPACT ANALYSIS FOR MIDDLETOWN DOLLAR GENERAL STORE

Lake County, CA

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February 12, 2015



TRAFFIC IMPACT ANALYSIS FOR MIDDLETOWN DOLLAR GENERAL STORE

Lake County, California

EXECUTIVE SUMMARY

• **Project Description.** The project is a 9,100 sf retail store to be located along State Route 29 (SR 29) just east of the Lake County community of Middletown. The store will occupy a portion of a vacant site on the south side of SR 29 between the Wardlaw Street and Butts Canyon Road intersections, as shown in Figure 1. The store is across SR 29 from the Middletown Bible Church.

The Trip Generation forecast for this store has been based on consideration of trip generation rates published by the Institute of Transportation Engineers (ITE) in their publication *Trip Generation Manual 9th Edition*. The project is expected to generate approximately 583 daily trips on a weekday basis (i.e., ½ inbound and ½ outbound). Of that total 35 trips are expected during the a.m. peak hour and 62 trips will occur during the evening commute hour. Of the project's traffic 34% is expected to be drawn from the stream of traffic already using SR 29 by the site (i.e., "pass-by" trips). The site will be visited by 3 or 4 large trucks (STAA) each week, although single unit trucks will likely make deliveries each day. A loading dock is planned on the southern end of the site. Thirty on-site parking stalls will be available, as shown in Figure 2.

The site plan indicates that access to the site is proposed at a single driveway on SR 29. The driveway will roughly align with the existing driveway serving the Middletown Bible Church.

- Study Scope. The breadth of this traffic study was determined in consultation with Caltrans District 1 and Lake County Public Works Department staff. The study evaluates immediate and long term traffic impacts at site access during the a.m. and p.m. peak traffic hours. This study adheres to Caltrans guidelines for the preparation of traffic impact studies.
- Existing Setting. SR 29 is a major arterial route serving Lake County providing access to Napa County to the south and connecting Middletown with the communities around Clear Lake to the north. In the area of the project SR 29 is a conventional two lane highway.

Peak hour traffic operations were evaluated at the existing intersection on SR 29 where the Dollar General Store will take access. Today motorist waiting to turn onto SR 29 from the Middletown Bible Church parking lot experience delays that are indicative of LOS C in the a.m. peak hour and LOS B in the p.m. peak hour. However, in the morning a long westbound queue of traffic extends back from the Wardlaw Street traffic signal beyond the Middletown Bible Church parking lot during the period when area residents are dropping off students at

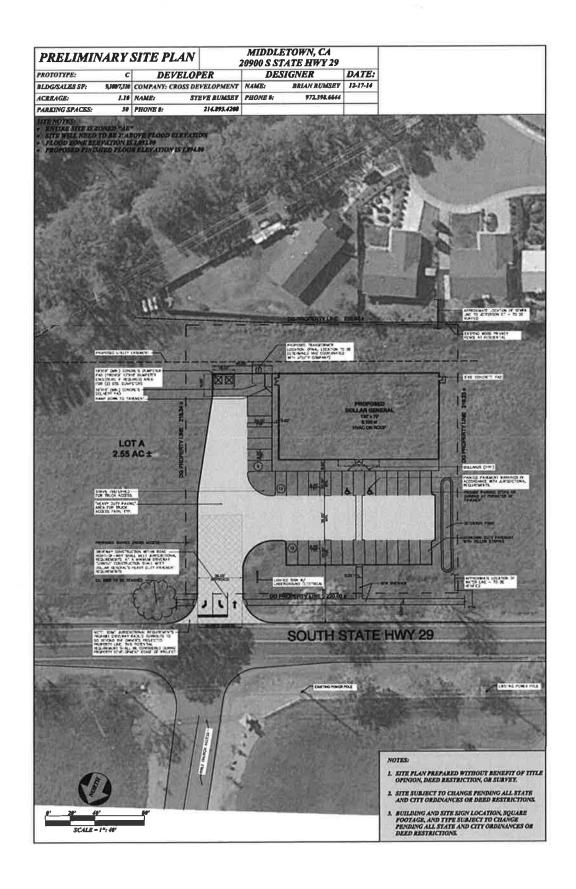


Middletown's school. The current Levels of Service for motorists exiting the church parking lot satisfy the Lake County minimum LOS C goal and the Caltrans TRC concept LOS E.

- Project Traffic Impacts on Existing Traffic Conditions. The project will add traffic to the area street system, but without improvements, study area intersections will still maintain acceptable Levels of Service in the vicinity of the project. The project will generate 35 trip ends in the a.m. peak hour and 62 trip ends in the p.m. peak hour.
- Project Impacts on Alternative Transportation Modes. The project may result in some pedestrians walking along SR 29. Sidewalk is planned as part of the project's frontage improvements, and in combination with the existing paved shoulders will provide adequate access. A marked crosswalk across SR 29 at the site access is not recommended.
- **Project Access Issues.** The volume of traffic at the project access will justify a westbound left turn lane based on AASHTO guidelines. The site access will need to accommodate truck turns. These improvements should accompany the project and be installed by the project proponents.
- Cumulative Plus Project Impacts. Long term traffic growth on SR 29 will make access from fronting properties difficult. Whether the Dollar General Store proceeds or not, a continuous Two-Way-Left-Turn lane will be needed, and with this improvement the project's access will deliver an adequate Level of Service. The TWLT lane will eventually benefit all the properties along SR 29, and the Dollar General Store project should contribute its fair share to the cost of a TWLT lane by installing its westbound left turn lane, which is part of the TWLT lane.
- With implementation of identified improvements the traffic impacts of the Dollar General Store are less than significant.







INTRODUCTION

Study Purpose and Objectives

This study evaluates the traffic impacts associated with developing a free-standing 9,100 sf Dollar General Store proposed on the south side of State Route 29 on the east side of the Lake County community of Middletown.

This study adheres to Caltrans traffic study guidelines and direction from Lake County Public Works Department staff. This study addresses the following scenarios, and considers conditions occurring during the a.m. and p.m. peak hour periods:

- 1. Existing traffic conditions in Year 2014;
- 2. Existing Plus Dollar General Store conditions;
- 3. Future Cumulative (Year 2035) conditions as presented in the SR 29 Transportation Concept Report (TCR) without the project; and,
- 4. Year 2035 conditions with the Dollar General Store.

The objectives of this study are:

- To identify whether the site access intersection will operate with minimum Levels of Service when the Dollar General Store is operating.
- To evaluate the adequacy of site access, with specific consideration of the need for a left turn lane on SR 29.
- To evaluate the adequacy of internal circulation, with specific consideration of the path of delivery trucks.
- To evaluate the adequacy of bicycle and pedestrian facilities in this area of the city.
- To evaluate long term impacts within the context of long term traffic conditions assuming development under the Lake County General Plan and regional traffic growth.

Project Description

Dollar General Stores is a chain of small to medium sized convenience oriented discount stores that are prevalent on the east coast but have only recently appeared in California. While store hours vary from store to store, this Dollar General Store is expected to be open from 7:00 a.m. to 11:00 p.m.

The Dollar General Store will occupy a 2½ acre portion of a vacant site on the south side of SR 29 between Wardlaw Street and Butts Canyon Road, as shown in Figure 1. The store is across the Middletown Bible Church.



The site plan (Figure 2) indicates that access to the site is proposed at a new driveway on SR 29. The driveway will roughly align with the existing driveway serving Middletown Bible Church and is 1,200 feet from the signalized Wardlaw Street intersection to the west and 950 feet from the Butts Canyon Road intersection to the east.

The project parking lot provides 30 parking spaces and includes a truck loading area. The truck loading area is at the south end of the site. Trucks would enter from SR 29, move along the front of the building and position themselves facing the SR 29 exit. Project proponents indicate full size trucks will make deliveries to the site three or four times a week and that STAA sized trucks could be involved.

EXISTING SETTING

Study Area Streets

This study addresses traffic conditions on state highways and Lake County roads in the vicinity of the proposed project within a study area identified in consultation with Caltrans District 1 and Lake County Department of Public Works staff. The text that follows describes the facilities included in this analysis.

Regional access to Middletown is provided by State Route 29 and to a lesser degree State Route 175 and Butts Canyon Road. Access to the Dollar Store will be via SR 29. The text that follows describes these existing facilities.

Functionally, study area streets are classified as Arterials, Collectors or Local Streets. The applicable designation is presented in the State Route 29 Transportation Concept Report (TCR), Lake County Regional Transportation Plan and Lake County General Plan Circulation Element.

State Route 29 (SR 29). SR 29 begins at the Napa/Lake County line and continues north through the community of Middletown to the community of Lower Lake and then proceeds north-west through the community of Kelseyville and the City of Lakeport to its terminus on SR 20 in Lower Lake. The southern portion of SR 29 is classified a Rural Minor Arterial road. In the area of the proposed Dollar General Store SR 29 is a two lane conventional highway with two 12' travel lanes and 4' paved shoulders. The Robert H. Weatherman Memorial Bridge across the Saint Helena Creek is located roughly 500 feet east of the proposed project access.

The California Department of Transportation (Caltrans) regularly monitors the volume of traffic on state highways. The most recent Caltrans traffic counts (2013) reveal that SR 29 carries an *Annual Average Daily Traffic (AADT)* volume of 11,500 vehicles per day in the area between the SR 175 junction and Butts Canyon Road. A 24 hr count conduced for this study on January 27, 2015 reported 11,155 vehicles on SR 29 west of the Middletown Bible Church access.

Trucks comprise 3% of the daily traffic volume on the portion of SR 29 near the project. The portion of SR 29 between the Napa County line and SR 175 in Middletown is a California Legal Advisory Road. The portion of SR 29 north of SR 175 to SR 20 is an STAA Terminal Route.

The posted speed limit on SR 29 is 45 mph in the area of the proposed Dollar General Store.

Bicycle and Pedestrian Facilities

Sidewalks exist today along Middletown's streets in the area west of the Wardlaw Street intersection. There are no sidewalks in the immediate area of the proposed Dollar General Store. There are crosswalks across SR 29 at Wardlaw Street and at the other intersections on SR 29 through Middletown. The existing paved shoulder along SR 29 in the area of the project is considered to be adequate for pedestrian traffic by Caltrans.



Public Transit

Lake Transit provides bus routes, regional flex route service and local dial-a-ride services within Lake County. One Lake Transit bus route utilizes SR 29 in the area of the project. Bus Route 3 which originates in the City of Clearlake uses SR 29 from Lower Lake to the Lake/Napa County line. Service is provided Monday through Saturday with five trips per day in each direction (four on Saturday).

Study Area Intersections

The limits of this analysis were identified in consultation with Caltrans District 1 and Lake County staff. Based on the amount of vehicular traffic associated with the proposed project and their understanding of traffic conditions in the Middletown area, this study focusses on the SR 29 / Middletown Bible Church Access intersection, as this will also be access to the Dollar General Store.

The SR 29 / Middletown Bible Church Access intersection is a 'tee" intersection controlled by an implied stop sign on the church parking lot exit. There is a single lane in each direction on SR 29 and on the church access. There are no auxiliary turn lanes at this intersection. A street light exists on the northeast corner of the intersection.

Level of Service Analysis Methodology / Thresholds of Significance

Methodology. The 2000 Highway Capacity Manual was used to provide a basis for describing the quality of existing traffic operating conditions and for evaluating the significance of project traffic impacts based on operating Level of Service. Level of Service (LOS) measures the quality of traffic flow and is represented by letter designations from "A" to "F", with a grade of "A" referring to the best conditions, and "F" representing the worst conditions. Table 1 presents typical Level of Service characteristics.



TABLE 1 LEVEL OF SERVICE DEFINITIONS

Level of Service	Signalized Intersection	Unsignalized Intersection	Roadway (Daily)
"A"	Uncongested operations, all queues clear in a single-signal cycle. Delay ≤ 10.0 sec		Completely free flow.
"B"	Uncongested operations, all queues clear in a single cycle. Delay > 10.0 sec and ≤ 20.0 sec	Short traffic delays. Delay > 10 sec/veh and < 15 sec/veh	Free flow, presence of other vehicles noticeable.
"C"	Light congestion, occasional backups on critical approaches. Delay > 20.0 sec and ≤ 35.0 sec	Average traffic delays. Delay > 15 sec/veh and ≤ 25 sec/veh	Ability to maneuver and select operating speed affected.
"D"	Significant congestion of critical approaches but intersection functional. Cars required to wait through more than one cycle during short peaks. No long queues formed. Delay > 35.0 sec and ≤ 60.0 sec	Delay > 25 sec/veh and	Unstable flow, speeds and ability to maneuver restricted.
"E"	Severe congestion with some long	extreme congestion. Delay > 35 sec/veh and ≤ 50 sec/veh	At or near capacity, flow quite unstable.
"F"		Intersection blocked by external causes. Delay > 50 sec/veh	Forced flow, breakdown.

Level of Significance. Caltrans employs various minimum Level of Service standards for its facilities depending on the type of facility and the characteristics of the location. Caltrans general minimum standard of LOS C is noted in Caltrans' Traffic Study Guidelines, but exceptions to that standard are documented in various planning and policy documents. In the case of SR 29, the 2013 SR 29 Transportation Concept Report (TCR) identified LOS E as the Concept Level of Service for SR 29 in the Middletown area.

The Lake County General Plan Policy T 1.8 identifies LOS C as the County's goal for its street but recognizes that LOS E may be accepted in locations where measures to provide LOS C are deemed infeasible due to cost, negative community and/or environmental impacts, and constructability issues.



The conditions described using Levels of Service vary for different types of intersections. Where traffic signals or all-way stops are installed, the Level of Service is based on the length of delays experienced by motorists stopped at the intersection, and overall average Level of Service is considered. At unsignalized intersections controlled by side street stop signs, individual Levels of Service can be determined for all motorists who must yield the right of way.

Existing Traffic Operating Conditions

Traffic Volumes. Current a.m. and p.m. peak hour turning movement counts were made at the study intersection during the a.m. peak hour (7:00 to 9:00 a.m.) and p.m. peak hour (4:00 to 6:00 p.m.) on January 27, 2015.

Figure 3 identifies the current intersection lane configuration used for the Level of Service analysis, as well as the results of the peak hour turning movement counts.

Intersection Levels of Service. Table 2 summarizes current Levels of Service at the study area intersections during the highest volume hour within each analysis period. As shown, the current Level of Service for traffic waiting to enter SR 29 is LOS C in the a.m. peak hour and LOS B in the p.m. peak hour. These conditions are within the County's LOS C goal and the TCR goal of LOS E.

EXIST	ING PEAK	TAB HOUI	LE 2 R LEVELS OF	SERV	ICE	
		AN	1 Peak Hour	PN	1 Peak Hour	
			Average Delay		Average Delay	Traffic Signal
Location	Control	LOS	(sec/veh)	LOS	(sec/veh)	Warranted?
SR 29 / Middletown Bible Church						
SB left + right turn	SB Stop	С	16.0	В	10.4	No

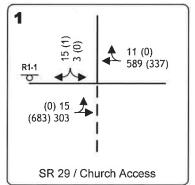
It is important to note that during the a.m. peak hour short term congestion associated with travel to Middletown schools and regional commute traffic results in appreciable congestion at the signalized intersection in the community. During that time period the queue of westbound traffic on SR 29 extends back from the Wardlaw Street intersection through the Middletown Bible Church driveway. Level of Service calculations do not reflect the delays associated with queueing from adjoining intersections.

Traffic Signal Warrants. As noted in Table 2, the intersection does not carry traffic volumes that reach a level that would satisfy peak hour warrants for signalization.









EXISTING TRAFFIC VOLUMES AND LANE CONFIGURATIONS

Left Turn Channelization. The American Association of State Transportation and Highway Officials (AASHTO) has identified guidelines for the installation of left turn lanes in their publication *A Policy on Geometric Design of Highways and Streets*. These guidelines, which are presented in their Exhibit 9-75 and Table 3 below, base the need for a left turn lane on the volume of traffic on the mainline road and the relative percentage of that traffic that turns. These criteria are applicable to intersections where the major street traffic proceeds freely and side street traffic is controlled by stop signs.

Review of weekday a.m. and p.m. peak hour volumes at the Middletown Bible Church access reveals that the greater number of left turns occurs during the a.m. peak hour. As noted in Table 3, the current combination of advancing and opposing volumes occurring during the a.m. peak hour traffic falls below the level that would justify a separate left turn lane at 40 or 50 mph.

7	FRAFFIC VOLUM	TABLE 3 ES JUSTIFYING	LEFT TURN LAN	ES
Opposing		Advancing Vo	olume (veh/hr)	
Volume (veh/hr)	5% Left Turns	10% Left Turns	20% Left Turns	30% Left Turns
	4	0-mph operating spe	ed	
800	330	240	180	160
600	318 (5%)	<u> </u>	8	
600	410	305	225	200
400	510	380	275	245
200	<mark>640</mark>	470	350	305
100	720	515	390	340
	5	0-mph operating spe	ed	
800	280	210	165	135
600	318 (5%)	2	2	: <u>*</u>
600	350	260	195	170
400	430	320	240	210
200	550	400	300	270
100	615	445	335	295
	6	0-mph operating spe	ed	
800	230	170	125	115
600	290	210	160	140
400	365	270	200	175
200	450	330	250	215
100	505	370	275	240

Source: A Policy on Geometric Design of Highway and Streets, AASHTO, 2004.

Existing AM Peak Hour at SR 29 / Middletown Bible Church



PROJECT CHARACTERISTICS

The relative impacts of developing the Dollar General Store and the adequacy of site access is dependent on the physical characteristics of the adjoining street system, as well as the amount of traffic generated by the proposed project. The amount of additional traffic on a particular section of the street network is dependent upon two factors:

- I. Trip Generation, the number of new trips generated by the project, and
- II. <u>Trip Distribution and Assignment</u>, the specific routes that the new traffic takes.

Trip Generation

This analysis considered trip generation rates derived from several sources. The Institute of Transportation Engineers (ITE) publication "Trip Generation, 9th Edition" provides information on the characteristics of various retail uses. The use most similar to Dollar General Store is "Variety Store" (Code 813). This information is based on surveys prepared for the Florida Department of Transportation (FDOT) in early 2011. The land use description notes that a Variety Store is a retail store providing health care & beauty aids, cleaning supplies, snack food, household items and some apparel. This is not a "dollar store" where everything is priced at one dollar, but rather is a small neighborhood store offering value and convenience. The stores studied were free-standing and catered to the local neighborhood. The 15 sites studied had building floor areas that ranged from roughly 8,000 to 17,000 square feet. Table 4 identifies the trip generation rates reported by ITE.

TABLE 4 TRIP GENERATION RATES												
		A	M Peak Ho	ur	P	M Peak Ho	ur					
Land Use / Source	Unit	In	Out	Total	In	Out	Total					
Variety Store	ksf	50%	50%	3.81	50%	50%	6.82					
Middletown Dollar General Store	9.1 ksf	18	17	35	31	31	62					
Pass-By Trips	34%	<6>	<6>	<12>	<11>	<10>	<21>					
Net New Trips		12	11	23	20	21	42					

Trip Generation Forecasts. Table 4 displays the a.m. and p.m. peak hour trip generation forecasts for the 9.1 ksf Dollar General Store. As shown, a portion of the traffic drawn to these stores would be drawn from the stream of traffic already passing each site. Customer surveys conducted for the FDOT study revealed that on average 34% of the weekday trips were "pass-by". This rate is comparable to the average pass-by rates reported by ITE for all shopping centers (i.e., also 34%).



As noted in Table 4, the project is expected to generate 23 new trips during the a.m. peak hour, with 42 new trips occurring during the p.m. peak hour.

The volume of traffic generated by variety stores is highest at midday and during the evening commute period. On a daily basis, these stores generate 64.03 trips per ksf. After discount for "pass-by trips", the proposed project may generate 384 new daily trips (½ inbound and ½ outbound).

Truck Trips. The proposed project will receive regular deliveries from the Dollar General Stores regional distribution center serving this area of California. Project proponents anticipate that 3 full size trucks will visit the store each week, although smaller single unit trucks may visit each day. Some of the full size trucks are expected to be STAA trucks (53') permitted on California highways under the Surface Transportation Authorization Act.

Truck Turning Requirements. These issues are important with regards to truck circulation. The project will result in full size trucks (STAA) turning into and out of the site via the project's SR 29 access intersection. The turning requirements of large trucks (i.e., STAA trucks) will need to be reviewed when final plans are prepared.

Trip Distribution

The distribution of project traffic was determined based on knowledge of the demographic distribution of residences in the south Lake County area and on market characteristics of Dollar General Stores. As noted in Table 5, the majority of the new trips attracted to the site will arrive from the west and a lesser share will arrive from the east. Pass-by trips will be drawn from passing in proportion to the volume of traffic during each time period.

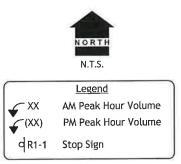
		ABLE 5 AP DISTRIBUTION	N	
		Pe	rcentage of All Trip	os
			Pass	-By
Direction	Route	New	AM	PM
East	SR 29	40%	黨	25
West	SR 29	60%	<u></u>	180
Eastboun	d on SR 29	-	33%-	67%
Westboun	d on SR 29		67%	33%
	Total	100%	100%	100%

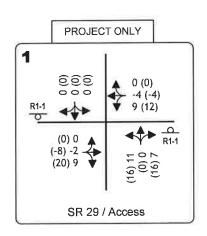
Trip Assignment

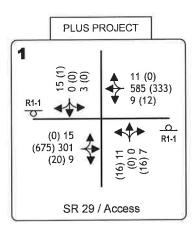
Project trips were assigned to the adjacent street, and Figure 4 illustrates the projected "Dollar General Store Traffic Only" traffic volumes forecast for the a.m. and p.m. peak hours.











EXISTING PLUS PROJECT TRAFFIC VOLUMES AND LANE CONFIGURATIONS

PROJECT IMPACTS

Opening Day Plus Project Traffic Conditions

The impacts of operating the proposed project have been identified by superimposing project trips onto the existing background condition. Resulting intersection Levels of Service were then calculated and used as the basis for evaluating potential project impacts. Figure 4 also presents the "Existing Plus Project" traffic volumes used for this analysis.

Intersection Levels of Service. Table 6 compares Existing and Plus Project peak hour Levels of Service and average delay per vehicle at the access intersection. As shown, motorists entering SR 29 will experience delays that are characteristics of LOS C conditions. However, as noted, delays may be longer in the a.m. peak hour due to short term congestion created at traffic signals in Middletown.

EXISTING P	LUS PRO		TABLE PEAK F	_	LEVELS	OF SI	ERVICE						
AM Peak Hour PM Peak Hour													
		Ex	isting	Ex Pl	us Project	E	cisting	Ex Pl	us Project				
			Average		Average		Average		Average				
			Delay		Delay		Delay		Delay				
Location	Control	LOS	(sec/veh)	LOS	(sec/veh)	LOS	(sec/veh)	LOS	(sec/veh)				
SR 29 / Middletown Bible Church	AID OD												
SB left + right turn	NB-SB	С	16.0	С	17.1	В	10.4	В	10.4				
NB left + right turn	Stop	N=1	355	C	24.1			С	23.6				

Left Turn Lane Channelization. The combination of opposing and advancing vehicles at the project access has been reviewed and compared to AASTO guidelines to determine whether a separate westbound left turn lane is justified. The greatest number of left turns will be expected during the p.m. peak hour, and at that time the 12 anticipated left turns would equal 4% of the total advancing traffic. As shown in Table 7, the forecast traffic volumes fall within the range that suggests a separate left turn lane is needed.



Opposing	PLUS PROJECT TR	Advancing Vo		
Volume (veh/hr)	5% Left Turns	10% Left Turns	20% Left Turns	30% Left Turns
		40-mph operating spe	ed	
800	330	240	180	160
695	345 (4%)	2		
600	318 (5%)	â		-
600	410	305	225	200
400	510	380	275	245
200	640	470	350	305
100	720	515	390	340
		50-mph operating spe	ed	
800	280	210	165	135
695	345 (4%)	-	5	
<mark>600</mark>	318 (5%)	¥	-	·
600	350	260	195	170
400	430	320	240	210
200	550	400	300	270
100	615	445	335	295
		60-mph operating spe	ed	
800	230	170	125	115
600	290	210	160	140
400	365	270	200	175
200	450	330	250	215
100	505	370	275	240

Source: A Policy on Geometric Design of Highway and Streets, AASHTO, 2004.

Existing AM Peak Hour at SR 29 / Middletown Bible Church

Existing PM Plus Project at Project Access

Design Issues. Caltrans typically designs left turn lanes to accommodate both waiting vehicles and the deceleration requirements of vehicles making turns. HDM Table 405.2b notes the distance recommended for a vehicle to come to a stop at various speeds. For a 45 mph design a distance of 375 feet is noted, although the HDM does permit designs that assume partial deceleration in the travel lane prior to entering the left turn lane. A portion of SR 49 east and west of the left turn lane will also need to be widened to accommodate the transition area. Assuming that widening occurs on both north and south sides of the highway intersection, a transition length of 270 feet would be needed to accommodate 6 feet of widening in each area.



The distance from the access to the adjoining bridge will play a role in the design of the left turn lane. Roughly 515 feet could be available from the end of a left turn lane and the guardrail on the north side of SR 29. Within that distance the work will need to accommodate the transition area (i.e., 270 feet), a bay taper into the new lane (90 feet) and a left turn lane that is roughly 155 feet long. The sum of turn lane and bay taper is 245 feet, which would satisfy HDM requirement for deceleration from 30 mph to a stop. A 270 foot long transition will be needed west of the intersection to bring the travel lanes back to centerline.

Impacts to Non-Automotive Transportation Modes

Pedestrian Impacts. The proposed project could attract pedestrians from Middletown. While the project frontage may include sidewalk, these persons would be expected to walk along the existing paved shoulder to and from Middletown and to cross SR 29 at a controlled location in town.

It is possible that pedestrians may occasionally wish to walk across SR 29 from the Middletown Bible Church, although pedestrian activity would be relatively rare. Marked crosswalks can have the unintended consequence of providing pedestrians with a false sense of security. Because pedestrians may incorrectly assume that drivers will stop due to the crosswalk, a marked crossing at the project access is not recommended.

Bicycle Impacts. Similarly, the project may attract bicycle traffic from Middletown. While there are no designated bicycle facilities, the existing paved shoulder will be adequate for this purpose.



CUMULATIVE IMPACTS

Based on Caltrans traffic study guidelines, this analysis considers two cumulative traffic conditions:

- 1. Existing Plus Approved or Pending Projects (EPAP), and
- 2. Year 2035 Conditions as projected in the SR 29 TCR.

Existing Plus Approved / Pending Projects (EPAP) Conditions

Approved Projects. The status of other development projects in the Middletown area was discussed with Lake County and Caltrans staff. As no approved / pending projects were identified, the Existing Plus Approved Project scenario was not investigated for this analysis.

Year 2035 Cumulative Impact Analysis

Available data suggests that the volume of traffic on SR 29 will increase in the future as regional traffic continues to increase. Table 8 compares daily traffic volumes on SR 29 reported by Caltrans. Comparison of 1993 and 2013 volumes reveals that daily traffic grew at a rate of about 2.1% annually over this twenty year period. Assuming that the TCR Horizon year is 20 years from 2013, the projected growth rate over this period is 2.4% annually.

GROV	TABI VTH TRE	LE 8 NDS ON SR	29		
		Annua	l Average Dai	y Traffic	
Location	1002	2002	2013	2015	TCR
	1993	2003	2013	2015	Horizon
SR 29 between SR 175 and Butts Canyon Rd	7,500	9,100	11,500	11,155	18,360

Long Term Improvements. The extent of potential improvements to SR 29 in the study area was determined from review of the TCR and discussion with Caltrans staff. No major capacity increasing projects are anticipated, and the TCR suggest that safety improvements are possible. Caltrans staff note that a continuous Two-Way-Left-Turn (TWLT) lane is the preferred strategy but that funding for this improvement is uncertain.

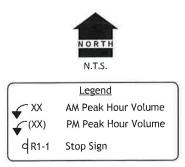
Year 2030 Intersection Levels of Service. Figure 5 identifies Cumulative Plus Project traffic volumes assuming that peak hour traffic on SR 29 increases by 2.4% annually for 20 years. Table 9 identifies Year 2030 intersection Levels of Service with and without the proposed Dollar General Store project. As shown, if background traffic increases as anticipated, then the Level of Service for motorists waiting to turn onto SR 29 from the Dollar General Store parking lot will deteriorate to LOS F. This would exceed the LOS E TCR goal.

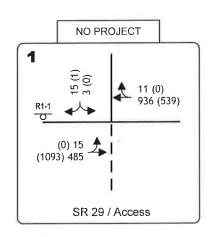


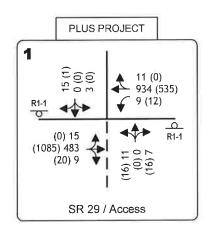
As also noted, installation of a continuous TWLT lane will improve the Level of Service by allowing motorists to make a "two-step" left turn. As shown LOS C and LOS D conditions are anticipated.

	00212	JLATI	TABLE 9 IVE PLUS LEVELS						
			AM Pea	k Hou	r		PM Pea	ık Hou	r
		Cui	mulative	Plus	Project	Cui	nulative	Plus	s Project
Location	Control	LOS	Average Delay (sec/veh)	LOS	Average Delay (sec/veh)	LOS	Average Delay (sec/veh)	LOS	Average Delay (sec/veh)
		Withou	ut Improve	ments					
SR 29 / Middletown Bible Church SB left + right turn NB left + right turn	NB-SB Stop	D -	27.0	D F	31.4 63.4	В	12.1	B F	12.1 63.5
		With	h TWLT L	ane					
SR 29 / Middletown Bible Church SB left + right turn NB left + right turn	NB-SB Stop	C -	22.0	D C	25.3 23.9	В	12.1	B D	12.1 28.3









CUMULATIVE PLUS PROJECT TRAFFIC VOLUMES AND LANE CONFIGURATIONS

MITIGATION SUMMARY

The Dollar General Store project should be responsible for constructing a westbound left turn lane on SR 29 in the area between the project access and the bridge. This improvement would address the relative need for a left turn lane under AASHTO guidelines. This improvement would also represent the project's "fair share" contribution towards the need for a continuous TWLT lane on SR 29 to accommodate long term cumulative traffic.

The Dollar General Store should also be responsible for widening its SR 29 access to accommodate the turning paths of trucks.

With these improvements the impacts of the Dollar General Store project would be considered to be less than significant.

APPENDIX

ALL TRAFFIC DATA (916) 771-8700 orders@aldtraffic.com

City of Middletown All Vehicles on Unshifted Nothing on Bank 1 Nothing on Bank 2

Unshifted Count = All Vehicles

7240-06

File Name © 15-7078-001 SR 29-Middletown Bible Church Driveway.ppd Date © 1/27/2015

	Jum Total	0006	0	0000	0	000	000	00	000	o (9															
	Total	154 159 256	867	211 171 179	706	225	249	250	198	2455	100,0%		Total		256	211	936	.785		Total		249	250	231	1021	877
'ay	APP TOTAL	2000	7 72	ω + + α	2	6040	2-6	00	0 ++		%6.0	/ay	APP TOTAL		N 5	[⊻] თ .•	- 82	375	vay	APP TOTAL		*	00	0	~	.250
Middletown Bible Church Driveway	RIGHT DTURNS	0000	0	0000	0	000	000	00	000	5	%0°0	Middletown Bible Church Driveway Eastbound	RIGHT UTURNS		0 0	000	0 0	000	Middletown Bible Church Driveway Eastbound	TURNS		0	00	0	0.0%	000
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	LEFT	0000	101	-0-0	67	<u>ω</u> ← α	V O Ø	00	000	o	31.3%		EFF.	TW3	0.0	4-0	3 18 7%	375	L	- LEFT		0	00	0	0 %	8
	APP TOTAL	339	261	4 4 6 4 4 6 6 4	247	128	163	192	151	100	50.1%		APP TOTAL		7,	2 8 2	318	729		APP TOTAL		163	177	151	683	688
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	THRU	38 7 38 7 38	246	84 60 84	245	21 1 28 1 4 4 5	163	177	151	120	99 0%		THRU		71	8 8 8	303	787		Н		163	177	151	683	888
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pu	UTURNS	0000	0	0000	0	000	000	00	000	o 0	%00	pu	RIGHT UTURNS APP TOTAL		00	000	0 0	000	pu	RIGHT [UTURNS] APP TOTAL		0	00	0	0.0%	000
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	THRU	0000	0	0000	0	000	000	00	000	o	%0'0		THRU		00	00	0 0	000		THRU		0	00	0	0.0%	000
		0000	0	0000	0	000	000	00	000		0.0%		I LEFT		00	0.6	0 0	000	_	LEFT	1	0	00	0	0.0	000
	APP TOTAL	112 120 183 77	269	124 116 118	452	77	888	73	89 89	0 0 0	48.9%		APP TOTA		183	124	009	.820		APP TOTA			E 8			.851
700	RIGHT [UTURNS] APP TOT	0000	0	0000	0	000	000	00	000	o	%0.0	- שטי	THRU RIGHT UTURNS APP TOT	at 07:30	00	000	0 %	000	pur	UTURNS	at 16:45	0	00	0	0 0	000
Southbound	RIGHT	0446	1	o-00	4	-00	0 -	00	0 - 1	T 4	1.0%	SR 29	RIGHT	to 08:30	40	າ ຕຸ	11 2%	889	SR 29	RIGHT	to 17:45	0	00	0	0.0%	000
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	LEFT	0000		0000		000			000		0 0		HEI!	nalysis F or Entire		000	C			HEI.	unalysis F	0	0 0	0	e 0 0%	4
	START TIME	07.00 07.15 07.30	Total	08:00 08:15 08:30 08:45	Total	16:00	16:45 Total	17:00	17:30	Total Total	Approh % Total %	AM PEAK HOUR	START TIME	Peak Hour Analysis From 07:30 to 08:30 Peak Hour For Entire Intersection Begins at 07:30	07.30	080	Total Volume	出	PM PEAK HOUR	START TIME LEFT (THRU RIGHT UTURNS APP TOT	Peak Hour A	16:4	17:00 0 73 0 0 17:15 0 99 0 0	17:30	Total Volum	44

Location		of Middletov	Wit profit City	T	C	VICTORIAN CO.	1.1	T	7240-06	nd Totals
Stare Time	Morning	Afternoon	Morning	Totals Arternoon	Morning	Atternoon	Marning	Attornaon	Marning	Arternoa
12:00	13	75			3	84				
12:15	17	85			4	72				
12:30	15	82			8	76				
12:45	8	69	53	311	î	68	16	300	69	611
1:00	10	97	(9000)	30,31,7	3	90	1.50	(4.5.5)	72.45	
1:15	2	82				85				
1:30	3	84			2	81				
1:45	3	71	18	334	2 2 2	90	9	346	27	680
	8	90	10	334	ō	100		5.10	fo.f.	COL
2:00					4	72				
2:15	14	126								
2:30	0	126	no.	400	4	99	4.4	205	70	700
2:45	7	88	29	430	3	94	11	365	40	795
3:00	2 6	114			4	100				
3:15	6	142			4	102				
3:30	2	108			12	108				
3:45	4	135	14	499	12	94	32	404	46	903
4:00	8	125			11	87				
4:15	6	147			16	80				
4:30	7	136			32	84				
4:45	5	165	26	573	40	85	99	336	125	909
5:00	6	171			41	74				
5:15	5	197			52	103				
5:30	9	143			85	79				
5:45	17	139	37	650	101	65	279	321	316	971
6:00	19	132	57	.000	103	59	, 60			34.7.1
6:15	30	135			117	58				
6:30	32	113			101	44				
	40	83	121	463	108	37	429	198	550	661
6:45			121	403		41	463	130	330	001
7:00	40	62			111					
7:15	.39	71			118	32				
7:30	62	45	2222	20000	180	35	con	***	oro	201
7:45	118	60	259	238	184	35	593	143	852	381
8:00	81	59			128	26				
8:15	60	38			115	24				
8:30	54	40		::=xn	119	20		(426)	525520	50.01
8:45	55	39	250	176	97	11	459	81	709	257
9:00	61	35			82	17				
9:15	64	37			93	13				
9:30	65	28			100	18	0			
9:45	43	26	233	126	89	15	364	63	597	189
10:00	45	19		2-250	92	4		100.000		
10:15	64	29			99	9				
10:30	58	30			89	6				
10:45	61	14	228	92	103	5	383	24	611	116
11:00	66	28	111111111111111111111111111111111111111	2.0	89	7	550.00		13712	
11:15	87	11			81	18				
11:30	57	13			80	9				
	85	11	295	63	86	12	336	46	631	109
11:45	1563	3955	1563	3955	3010	2627	3010	2627	4573	6582
Total										
n bined	551	8	55	18	56	37	56	37	111	155
Total					7.15 414					
1 Punk	11:45 AM				7:15 AM					
Vot.	327				610					
P.H.F.	0.962	V 327201			0.829	0.45 54				
1 Pook		4:45 PM				2:45 PM				
Vot.		676				404				
P.H.F.		0.926				0.935				
12.4.194.5										

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Wed Feb 11, 2015 06:43:55 _____

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EXISTING PLUS PROJECT MIDDLETOWN DOLLAR GENERAL 7240-06

Scenario Report

Scenario: EX AM

Command:

Volume:
EX AM

Geometry:
Impact Fee:
Trip Generation:
Trip Distribution:
Paths:
Routes:
Configuration:
Default Command
EX AM
EXISTING
Default Impact Fee
CURRENT
Default Path
Default Route
Default Configuration

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EXISTING PLUS PROJECT MIDDLETOWN DOLLAR GENERAL 7240-06

Trip Generation Report

Forecast for AM PEAK

		-					
9.	10 RETAIL KSF	1.28	1.28	12	12	24	100.0
Zone 1 Subtot	al			12	12	24	100.0
	200	9.10 RETAIL KSF Zone 1 Subtotal	7.0 min - 00000000000000000000000000000000000	9.10 RETAIL KSF 1.28 1.28 Zone 1 Subtotal			3110 1011111 101

EXISTING PLUS PROJECT MIDDLETOWN DOLLAR GENERAL 7240-06

Turning Movement Report AM PEAK

Volume	N	orthbo	ound	S	outhb	ound	E	astbo	und	W	estbo	und	Total
Type	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Volume
#1 SR 2	29 / 1	Access	5										
Base	0	0	0	3	0	15	15	303	0	0	589	11	936
Added	7	0	5	0	0	0	0	0	7	5	0	0	24
PASS B	4	0	2	0	0	0	0	-2	2	4	-4	0	6
Total	11	0	7	3	0	15	15	301	9	9	585	11	966

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EXISTING PLUS PROJECT MIDDLETOWN DOLLAR GENERAL 7240-06

> Impact Analysis Report Level Of Service

Intersection

1 SR 29 / Access

Base Future Change
Del/ V/ Del/ V/ in
LOS Veh C LOS Veh C
C 16.0 0.047 C 24.1 0.095 + 8.082 D/V

EXISTING PLUS PROJECT MIDDLETOWN DOLLAR GENERAL 7240-06

Level Of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) ************ Intersection #1 SR 29 / Access ************** Average Delay (sec/veh): 0.5 Worst Case Level Of Service: C[16.0] ************************* Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R Movement: _____| Control:Stop SignStop SignUncontrolledUncontrolledRights:IncludeIncludeIncludeIncludeLanes:0 0 1! 0 0 0 0 1! 0 0 0 0 1! 0 0 0 0 1! 0 0 Volume Module: >> Count Date: 27 Jan 2015 << 15 303 0 0 589 11 Base Vol: 0 0 0 3 0 15 PHF Volume: 0 0 0 4 0 19 19 386 0 0 750 14 Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 FinalVolume: 0 0 0 4 0 19 19 386 0 0 750 0 Critical Gap Module: Critical Gp: 7.1 6.5 6.2 6.4 6.5 6.2 4.1 xxxx xxxxx xxxxx xxxx xxxxx _____ Capacity Module: Cnflict Vol: 1191 1189 386 1182 1182 662 210 190 757 764 xxxx xxxxx xxxx xxxx xxxx 407 849 xxxx xxxxx xxxx xxxx xxxx Potent Cap.: 164 188 Move Cap.: 154 184 662 206 185 407 849 xxxx xxxxx xxxx xxxx xxxx xxxx Volume/Cap: 0.00 0.00 0.00 0.02 0.00 0.05 0.02 xxxx xxxx xxxx xxxx xxxx xxxx Level Of Service Module: C: Note: Queue reported is the number of cars per lane. **** _____

EXISTING PLUS PROJECT MIDDLETOWN DOLLAR GENERAL 7240-06

Level Of Service Computation Report 2000 HCM Unsignalized Method (Future Volume Alternative) *********** Intersection #1 SR 29 / Access ***************** Average Delay (sec/veh): 1.0 Worst Case Level Of Service: C[24.1] *************** Approach: North Bound South Bound East Bound West Bound L - T - R L - T - R L - T - R Movement:
 Control:
 Stop Sign
 Stop Sign
 Uncontrolled
 Uncontrolled

 Rights:
 Include
 Include
 Include
 Include

 Lanes:
 0 0 1! 0 0 0 0 1! 0 0 0 0 1! 0 0 0 0 1! 0 0
 0 0 1! 0 0
 0 0 1! 0 0
 Volume Module: >> Count Date: 27 Jan 2015 << 15 303 0 0 589 Base Vol: 0 0 0 3 0 15 Initial Bse: 0 0 0 3 0 15 15 303 0 0 589 11 Added Vol: 7 0 5 0 0 0 0 0 7 5 0 0 PASS BY: 4 0 2 0 0 0 0 -2 2 4 -4 0 Initial Fut: 11 0 7 3 0 15 15 301 9 9 585 11 11 745 _____| Critical Gap Module: Critical Gp: 7.1 6.5 6.2 7.1 6.5 6.2 4.1 xxxx xxxxx 4.1 xxxx xxxxx FollowUpTim: 3.5 4.0 3.3 3.5 4.0 3.3 2.2 xxxx xxxxx 2.2 xxxx xxxxx -----| Capacity Module: Cnflict Vol: 1212 1210 389 1207 1208 752 759 xxxx xxxxx 395 xxxx xxxxx Potent Cap.: 159 183 659 160 183 410 852 xxxx xxxxx 1164 xxxx xxxxx Move Cap.: 148 177 659 154 177 410 852 xxxx xxxxx 1164 xxxx xxxxx Volume/Cap: 0.09 0.00 0.01 0.02 0.00 0.05 0.02 xxxx xxxx 0.01 xxxx xxxx ______[______|_________| Level Of Service Module: 9.3 xxxx xxxxx Control Del:xxxxx xxxx xxxxx xxxxx xxxxx xxxx 8.1 xxxx xxxxx LOS by Move: * * * * * * * A * * A * * * Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT A * * ApproachLOS: XXXXXX C C ****************** Note: Queue reported is the number of cars per lane.

Traffix 8.0.0715 (c) 2008 Dowling Assoc. Licensed to kdANDERSON TRANSP.

EXISTING PLUS PROJECT MIDDLETOWN DOLLAR GENERAL 7240-06

Scenario Report

Scenario:

EX PM

Default Command

Volume: EX PM
Geometry: EXISTING
Impact Fee: Default Impact Fee
Trip Generation: PM PEAK
Trip Distribution: CURRENT
Paths: Default Path
Routes: Default Route

Configuration: Default Configuration

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EXISTING PLUS PROJECT MIDDLETOWN DOLLAR GENERAL 7240-06

Trip Generation Report

Forecast for PM PEAK

Zone #	Subzone	Amount	Units	Rate In	Rate Out	Trips In	Trips Out	Total Trips	% Of Total
1		9.10	RETAIL KSF	2.25	2,25	20	20	40	100.0
	Zone 1	Subtotal				20	20	40	100.0
TOTAL						. 20	20	40	100.0

EXISTING PLUS PROJECT MIDDLETOWN DOLLAR GENERAL 7240-06

Turning Movement Report PM PEAK

Volume	Nor	thbou	und	S	outhb	ound	E	astbo	und	W	estbo	und	Total
Туре	Left T	hru I	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Volume
#1 SR 2	29 / Ac	cess											
Base	0	0	0	.0	0	1	0	683	0	0	337	0	1021
Added	12	0	8	0	0	0	0	0	12	8	0	0	40
PASS B	4	0	8	0	0	0	0	-8	8	4	-4	0	12
Total	16	0	16	0	0	1	0	675	20	12	333	0	1073

EX	PM

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EXISTING PLUS PROJECT MIDDLETOWN DOLLAR GENERAL 7240-06

Impact Analysis Report Level Of Service

Intersection

1 SR 29 / Access

EXISTING PLUS PROJECT MIDDLETOWN DOLLAR GENERAL 7240-06

Level Of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) Intersection #1 SR 29 / Access ***************** Average Delay (sec/veh): 0.0 Worst Case Level Of Service: B[10.4] ******************* Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - RControl: Stop Sign Stop Sign Uncontrolled Uncontrolled Rights: Include Include Include Include Lanes: 0 0 1! 0 0 0 0 0 0 1 0 0 1 0 0 0 0 0 Volume Module: >> Count Date: 27 Jan 2015 << Base Vol: 0 0 0 0 0 1 0 683 0 0 337 0 Critical Gap Module: _____ Capacity Module: Cnflict Vol: 1164 1163 779 xxxx xxxx 384 xxxx xxxx xxxxx xxxx xxxx xxxx Volume/Cap: 0.00 0.00 0.00 xxxx xxxx 0.00 xxxx xxxx xxxx xxxx xxxx xxxx xxxx _____|__|__|__|__| Level Of Service Module: Control Del:xxxxx xxxx xxxxx xxxxx xxxx 10.4 xxxxx xxxx xxxxx xxxxx xxxxx xxxxx 0.0 xxxx xxxxx 7.2 xxxx xxxxx A * XXXXXX ApproachLOS: Note: Queue reported is the number of cars per lane.

EXISTING PLUS PROJECT MIDDLETOWN DOLLAR GENERAL 7240-06

Level Of Service Computation Report 2000 HCM Unsignalized Method (Future Volume Alternative) ********************* Intersection #1 SR 29 / Access **************** Average Delay (sec/veh): 0.8 Worst Case Level Of Service: C[23.6] ***** Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R L - T - R ______|___| _____| Volume Module: >> Count Date: 27 Jan 2015 << Base Vol: 0 0 0 0 1 0 683 0 0 337 33/ 0 Initial Bse: 0 0 0 0 0 1 0 683 0 0 337 Added Vol: 12 0 8 0 0 0 0 1 2 8 0 PASS BY: 4 0 8 0 0 0 0 -8 8 4 -4 Initial Fut: 16 0 16 0 0 1 0 675 20 12 333 -----| Critical Gap Module: Capacity Module: Cnflict Vol: 1189 1188 781 xxxx xxxx 380 xxxx xxxx xxxxx 792 xxxx xxxxx Potent Cap.: 165 188 395 xxxx xxxx 667 xxxx xxxx xxxx 828 xxxx xxxxx Move Cap.: 162 185 395 xxxx xxxx 667 xxxx xxxx xxxxx 828 XXXX XXXXX _____| Level Of Service Module: 2Way95thQ: xxxx xxxx xxxxx xxxx 0.0 xxxx xxxx xxxx 0.1 xxxx xxxxx Control Del:xxxxx xxxx xxxx xxxx xxxx 10.4 xxxxx xxxx xxxx 9.4 xxxx xxxxx LOS by Move: * * * * * B * * * A * * Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT 9.4 xxxx xxxxx Shared LOS: * C * * * * * A * ApproachLOS: 10.4 XXXXXX XXXXXX C В Note: Queue reported is the number of cars per lane.

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CUM AM

Wed Feb 11, 2015 06:50:47

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CUMULATIVE PLUS PROJECT MIDDLETOWN DOLLAR GENERAL 7240-06

Scenario Report

Scenario:

CUM AM

Command:

Volume:
CUM AM
Geometry:
EXISTING
Impact Fee:
Default Impact Fee
Trip Generation:
AM PEAK
Trip Distribution:
CURRENT
Paths:
Default Path

Routes:

Default Route

Configuration: Default Configuration

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Trip Generation Report

Forecast for AM PEAK

Zone #	Subzone	Amount	Units	Rate In	Rate Out	Trips In	Trips Out	Total Trips	

1		9.10	RETAIL KSF	1.28	1.28	12	12	24	100.0
	Zone 1	Subtotal				12	12	24	100.0
TOTAL						4.6	12	250000	100.0

Turning Movement Report AM PEAK

Volume	Non	rthb	ound	S	outhb	ound	E	astbo	und	W	estbo	und	Total
Type	Left 7	Phru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Volume
#1 SR	29 / Ac	cces	5										
Base	0	0	0	3	0	15	1.5	485	0	0	942	11	1471
Added	7	0	5	0	0	0	0	0	7	5	0	0	24
PassBy	4	0	2	0	0	0	0	-2	2	4	-4	0	6
Total	11	0	7	3	0	15	15	483	9	9	938	11	1501

CYTYSE	70 74

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CUMULATIVE PLUS PROJECT MIDDLETOWN DOLLAR GENERAL 7240-06

Impact Analysis Report Level Of Service

Intersection

Base Future Change
Del/ V/ Del/ V/ in
LOS Veh C LOS Veh C
D 27.0 0.076 F 63.3 0.248 +36.368 D/V

Change

1 SR 29 / Access

Level Of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) ***************** Intersection #1 SR 29 / Access ************ Average Delay (sec/veh): 0.4 Worst Case Level Of Service: D[27.0] ************** Approach: North Bound South Bound East Bound West Bound L - T - R L - T - R L - T - R_____ _____ Volume Module: Initial Bse: 0 0 0 3 0 15 15 485 0 0 942 11 PHF Volume: 0 0 0 4 0 18 18 591 0 0 1149 13 Reduct Vol: 0 0 0 0 4 0 18 18 591 0 0 1149 13 FinalVolume: 0 0 0 4 0 18 18 591 0 0 1149 13 Critical Gap Module: _____|___|__| Capacity Module: Cnflict vol: 1793 1790 591 1784 1784 1156 1163 xxxx xxxxx xxxx xxxx xxxxx Potent Cap.: 63 81 507 90 82 239 601 xxxx xxxxx xxxx xxxx xxxx xxxxx _____ Level Of Service Module: Shared LOS: * * * * D * * * * * * 27.0 XXXXXX ApproachDel: xxxxxx ApproachLOS: D Note: Oueue reported is the number of cars per lane.

			MIDDFF	STOWN I	ЮПТА	K GENEI	KAL /Z	40-06				
		т.	errel (of Sars	rice (Computa	stion I	Renori	F			
21	000 HCM									tive)		
*****	*****	***	****	****	****	*****	****	****	****	*****	****	*****
Intersection	#1 SR	29 /	Acces	3.8								
Average Dela	y (sec/	veh)	:	1.3	****	Worst	Case 3	Level	Of Se	rvice:	F[63	3.3]
	Nort			Sou	th Bo	ound	Ea	ast Bo	ound	We	est Bo	ound
Movement:	L -	T	- R	L -	- T	• R	L -	- T	– R	L -	- T	- R
]				[
Control:	Sto	p Si	gn	Şt	op S	ign ide	Und	contr	olled	Unc	contro	offed
Rights:		nclu	ıde		Incl	ıde		Inclu	ude	0 (incii	lae
Lanes:	. 0 0	1!	0 0	0 () 1!	0 0	U	J T;	0 0) U) T;	
Volume Module Base Vol:	e: 0	0	0	3	0	15	15	303	0	0	589	11
Growth Adi:		-	1.00		1.00	1.00		1.60	1.00		1.60	1.00
Initial Bse:		0	0	3	0	15	15	485	0	0	942	11
Added Vol:	7	Ö	5	0	0	0	0	0	7	5	0	0
PasserByVol:	4	0	2	0	0	0	0	-2	2	4	-4	0
Initial Fut:		0	7	3	0	15	15	483	9	9	938	11
User Adj:		.00	1.00	1.00	1.00	1.00		1.00	1.00		1.00	1.00
PHF Adj:	0,82 0		0.82		0.82	0.82		0.82	0.82		0.82	0.82
PHF Volume:	13	0	9	_	0	18	18	589	11		1144	13
Reduct Vol:		0	0		0	0	0	0	0	0	0 1144	0 1.3
FinalVolume:	13	0	9		0	18	18	589				
										1 1		'
Critical Gap		6.5	6,2	7-1	6.5	6.2	Δ 1	xxxx	xxxxx	4.1	xxxx	xxxxx
Critical Gp: FollowUpTim:		4.0	3.3		4.0				xxxxx			XXXXX
	1									11		
Capacity Mod				'								
Cnflict Vol:		811	594	1808	1809	1151	1158	xxxx	xxxxx	600	XXXX	XXXXX
Potent Cap.:		79	505	61	79	241			XXXXX			XXXXX
Move Cap.:		75	505	58	75	241			XXXXX			XXXXX
Volume/Cap:	0.25 0	00,0	0.02	0,06	0.00	0.08	0.03	XXXX	xxxx	0.01	XXXX	XXXX
vorume/cap.]					
Level Of Ser							0 1	WWW.	xxxxx	0 0	VVVV	xxxxx
2Way95thQ:	XXXX X					XXXXX			XXXXX			XXXXX
Control Del:		*	*	*	*	*	в			A		
LOS by Move: Movement:			- RT			- RT			- RT		- LTR	- RT
Shared Cap.:	XXXX	83	xxxxx								xxxx	XXXXX
SharedOueue:	xxxxx	1.0	xxxxx	xxxxx	05	XXXXX	XXXXX	XXXX	XXXXX	XXXXX	xxxx	xxxxx
Shrd ConDel:	ххххх б	3.3	xxxxx	xxxxx	31.4	xxxxx	xxxxx	xxxx	XXXXX	xxxxx	xxxx	XXXXX
Shared LOS:	*	F		*	D	*	*	*	*	*	*	*
ApproachDel:	- 6	3.3			31.4		X	XXXXX		X	XXXXX	
ApproachLOS:		F	Market and the se		D	VI 12-10 124 - 10-1	ere armene e	*	Andrews Andrews		* anarananan	
										*****	****	n. n. 用 用 用 用 用 用
Note: Queue .	reporte *****	ed is	****	number	OI C	ars per	*****	****	*****	****	****	*****

	×				
		*		ri T	

Scenario Report

Scenario:

CUM PM

Command:

Volume:

Geometry:

Impact Fee:

Trip Generation:

Trip Distribution:

Paths:

Routes:

Configuration:

Default Command

CUM PM

EXISTING

Default Impact Fee

PM PEAK

CURRENT

Default Path

Default Route

Configuration:

Default Configuration

Page 2	-	1
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Trip Generation Report

Forecast for PM PEAK

Zone #	Subzone	Amount	Units	Rate In	Rate Out	Trips In	Trips Out	Total Trips	% Of Total
1		9.10	RETAIL KSF	2.25	2,25	20	20	100	100.0
	Zone 1	Subtotal	******			20	20	40	100.0
TOTAL						. 20	20	40	100.0

Turning Movement Report PM PEAK

Volume	N	orthb	ound	S	outhb	ound	E	astbo	und	W	estbo	und	Total
Type	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Volume
#1 SR 2	29 /	Acces:	9										
Base	0	0	0	0	0	1	0	1093	0	0	539	0	1633
Added	12	0	8	0	0	0	0	0	12	8	0	0	40
PassBy	4	0	8	0	0	0	0	8	-8	4	-4	0	12
Total	16	0	16	0	0	1	0	1101	4	12	535	0	1685

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CUMULATIVE PLUS PROJECT MIDDLETOWN DOLLAR GENERAL 7240-06

______ Impact Analysis Report

Level Of Service

Intersection

1 SR 29 / Access

Base Future Change
Del/ V/ Del/ V/ in
LOS Veh C LOS Veh C
B 12.1 0.002 F 63.5 0.290 +51.468 D/V

			MIDDL	ETOWN	DOLLA	R GENEI	RAL 72	40-06				
20 20 20 20 20 20 20 20 20 20 20 20 20 2			Level (of Ser	vice (Computa	ation I	Report				
	2000 1	HCM U1	nsigna	lized ù	Method	d (Base	e Volum	ne Ali	ternat	ive)		
******	* * * * * *	****	*****	*****	****	*****	****	*****	****	****	****	*****
Intersection	****	****	*****	****								
Average Delay (sec/veh): 0.0 Worst Case Level Of Service: B[12.1]												
Approach:	No	rth Bo	ound - R	Sou	ath Bo		E		ound	We		ound
Movement:		_ 1	_ K									
Control:	St	top S	ign	St	top Si	lgn	Uno	contro	olled	Und		olled
Rights:			ıde			ıde		Inclu			L 0	
Lanes:	. 0 () I!	0 0			0 1		0 0				
Volume Module										1 1		'
Base Vol:	0	0	0	0	0	1	Ω	683	0	0	337	0
Growth Adi:		1.00	1.00		1.00	1.00	_	1.60	1.00	_	1,60	1.00
Initial Bse:		0	1.00	0	0	1		1093	0	0	539	0
		1.00	1.00		1.00	1.00		1.00	1.00	1.00	1.00	1.00
User Adj:		0.92	0.92		0.92	0.92		0.92	0.92		0.92	0.92
PHF Adj:	0.92		0.32	0.52	0.52	1	0.52		0.0	0	586	0
PHF Volume: Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
	0	_	0	0	0	1		1188	0	0	586	0
FinalVolume:										*		
Critical Gap				11			1 1			' '		
Critical Gp:			6.2	xxxxx	xxxx	6.2	xxxxx	xxxx	XXXXX	xxxxx	xxxx	XXXXX
FollowUpTim:				xxxxx						XXXXX		
	1											
Capacity Mod												
Cnflict Vol:		1774	1188	xxxx	XXXX	586	xxxx	XXXX	xxxxx	XXXX	xxxx	xxxxx
Potent Cap.:			229		xxxx		XXXX	xxxx	xxxxx	XXXX	XXXX	XXXXX
Move Cap.:			229		xxxx		XXXX	xxxx	xxxxx	xxxx	xxxx	XXXXX
Volume/Cap:					xxxx		xxxx	XXXX	xxxx	XXXX	XXXX	XXXX
				{								1
Level Of Ser												
2Way95thO:	xxxx	xxxx	xxxxx	xxxx	XXXX	0.0	XXXX	XXXX	XXXXX			XXXXX
Control Del:	xxxxx	XXXX	xxxxx	XXXXX	XXXX	12.1	XXXXX	XXXX	XXXXX	XXXXX	XXXX	XXXXX
LOS by Move:	*	*	*	*	*	В	*	*	*	*	*	*
Movement:	LT ·	- LTR	- RT	LT ·		- RT			- RT		- LTR	- RT
Shared Cap.:	xxxx	0	XXXXX	XXXX	xxxx	xxxxx	XXXX	xxxx	XXXXX	XXXX		XXXXX
SharedOueue::	XXXXX	xxxx	XXXXX	XXXXX	XXXX	XXXXX	XXXXX	XXXX	XXXXX	0.0	XXXX	XXXXX
Shrd ConDel:	xxxxx	xxxx	XXXXX	xxxxx	xxxx	xxxxx	XXXXX	XXXX	XXXXX	7.2		XXXXX
Shared LOS:	*		*	*	*	k	*	*	*	A	*	*
ApproachDel:	X	xxxxx			12.1		X	xxxxx		X	XXXXX	
AnnwagahIACa		*			В			*			*	
*****									*****	****	****	*****
Nota: Onelle	renor	ted i	s the :	number	of ca	ars pe	r lane					
*******	****	****	****	*****	****	****	****	* * * * *	*****	****	****	*****

			WIDDL	ETOWN I	OOLLAI	R GENER	RAL 72	40-06				
				of for	ri co (Computa	ation l	anori				
21	200 4	OM Day	rices (od W	othod vice (Ompute Ututuu	re Vol	ime A	- lternat	ive)		
	****	****	******	126U P	*****	(*****	****	****	****	****	k*****
Intersection	#1 SI	R 29	/ Acces	3.8								
******	****	****	*****	· * * * * * :	****	****	*****	****	*****	*****	****	****
Average Delay	y (se	c/veh): *****	1.3	****	Worst	Case :	Level	Of Ser	vice:	F[6:	3.5] *****
Approach:			ound			ound					est Bo	
Movement:	L ·	- T	- R	T, ·	T	- R	ь.	- T	- R		- T	
Control:	St	top S:	Ĺgn	St	top S:	ign	Und	contro	olled	Unc	contro	olled
Rights:		Incl	ıde		Incl	ıde		Incl	ıde		Inc⊥ı	ıde
Lanes:	0 (0 1!	0 0	0 (0 0	0 1	0 (0 0	1 0	0 :	L O	0 0
Lanes.												
Volume Module								600	0		227	0
Base Vol:	0		0	0		1		683	1 00	1 00	337	1 00
Growth Adj:							100	1.60		0.1.00	1.60	1.00
Initial Bse:		-	0	0	0	1	-	1093	0 12	8	039	0
Added Vol:	12	0	8	0	0	0	0		-8	4	-4	0
PasserByVol:			8	0	0	•	_	1101	-o 4	12	535	0
Initial Fut:		0	16	1 00	1 00	1 00	1.00				1.00	1.00
	1.00		1.00		1.00	1.00		0.92	0.92		0.92	0.92
	0.92		0.92	0.92	0.92	0.92		1197		13		0.02
PHF Volume:		0	17 0	0	0	0		0		0	0	0
Reduct Vol:			17	0				1197	-	13		0
FinalVolume:	17	U	11									
Critical Gap				, ,								
Critical Gap			6.2	xxxxx	xxxx	6.2	xxxxx	xxxx	XXXXX	4.1	xxxx	XXXXX
FollowIInTim:	3.5	4.0	3.3	xxxxx	xxxx	3.3	xxxxx	xxxx	XXXXX	2.2	xxxx	XXXXX
	1											
Capacity Modu												
Cnflict Vol:	1807	1807	1199	xxxx	xxxx	582	XXXX	XXXX	XXXXX	1201	XXXX	XXXXX
Potent Cap.:				XXXX			xxxx	xxxx	XXXXX			XXXXX
Move Cap.:			226	xxxx	xxxx	513	xxxx	XXXX	XXXXX			XXXXX
Waluma/Can:	0 29	0.00	0.08	XXXX	xxxx	0,00	XXXX	XXXX	XXXX	0.02	XXXX	XXXX
volume/cap.					 -							
Level Of Ser	vice	Modul	e:									
2Way95thQ:	XXXX	XXXX	XXXXX	XXXX	XXXX	0.0	XXXX	XXXX	XXXXX			XXXXX
Control Del:	xxxxx	XXXX	XXXXX	XXXXX	XXXX	12.0	XXXXX	XXXX	XXXXX			*****
LOS by Move:				*	*					_	*	
Movement:	LT ·	- LTR	- RT	LT	- LTR	- RT	LT	- LTR	- RT	LT		- RT
Shared Cap.:	XXXX	95	XXXXX	XXXX	XXXX	XXXXX	XXXX	XXXX	XXXXX	XXXX		XXXXX
SharedQueue:	XXXXX	1.5	XXXXX	XXXXX	XXXX	XXXXX	XXXXX	XXXX	XXXXX			
Shrd ConDel:	XXXXX	63.5	XXXXX	XXXXX	XXXX	XXXXX	XXXXX	****	*****	11.3 B		*****
Shared LOS:						*				_	XXXXX	
ApproachDel:		63.5			12.0		х	XXXXX *		Х	*	
ApproachLOS: *******	Lateral P. C.	F	التاسيان السياب السياب	+++++	B *****	*****	*****	****	*****	*****	****	*****
******	****	*****	. # h c	~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~	of a	are no	r lane					
Note: Queue	repor ****	****	*****	*****	****	*****	*****	****	*****	****	****	*****

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CUMULATIVE PLUS PROJECT WITH TWLT LANE

MIDDLETOWN DOLLAR GENERAL 7240-06

Scenario Report

Scenario:

CUM AM

Command:

Volume:

Geometry:

Impact Fee:

Trip Generation:

Trip Distribution:

Paths:

Routes:

Default Command

CUM AM

WB LEFT TURN

Default Impact Fee

AM PEAK

CURRENT

Default Path

Default Route

Configuration: Default Configuration

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2000	A

Trip Generation Report

Forecast for AM PEAK

Zone #	Subzone	Amount	Units	Rate In	Rate Out	Trips In	Trips Out	Total Trips	% Of Total
1	Zone 1		RETAIL KSF	1.28	1.28	12 12	12 12		100.0
TOTAL						. 12	12	24	100.0

Turning Movement Report

AM PEAK

Volume	Nor	thbo	ound	S	outhb	ound	E	astbo	und	W	estbo	und	Total
Type	Left 1	hru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Volume
#1 SR	29 / Ac	cess											Co coessica
Base	0	0	0	3	0	15	1.5	485	0	0	942	11	1471
Added	7	0	5	0	0	0	0	0	7	5	0	0	24
PassBy	4	0	2	0	0	0	0	-2	2	4	-4	0	6
Total	11	0	7	3	0	15	15	483	9	9	938	11	1501

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Impact Analysis Report

Level Of Service

Intersection

1 SR 29 / Access

CUMULATIVE PLUS PROJECT WITH TWLT LANE

MIDDLETOWN DOLLAR GENERAL 7240-06 Level Of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) ********** Intersection #1 SR 29 / Access **************** Average Delay (sec/veh): 0.4 Worst Case Level Of Service: C[22.0] *************** Approach: North Bound South Bound East Bound West Bound Movement: L - T - R L - T - R _____|___| Stop Sign Stop Sign Uncontrolled Uncontrolled Include Include Include Control: Include Increase Rights: Include Include Include 0 0 1! 0 0 0 0 1! 0 0 1 0 0 1 0 |-----| Volume Module: Base Vol: 0 0 0 3 0 15 15 303 0 0 589 0 Initial Bse: 0 0 0 3 0 15 15 485 0 942 PHF Adj: PHF Volume: 0 0 0 4 0 18 18 591 0 0 1149 13 Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 FinalVolume: 0 0 0 4 0 18 18 591 0 0 1149 13 _____ Critical Gap Module: _____| Capacity Module: Cnflict Vol: 1793 1790 591 1784 1784 1156 1163 xxxx xxxxx xxxx xxxx xxxxx 90 82 239 601 xxxx xxxxx xxxx xxxx xxxx xxxxx 88 79 239 601 xxxx xxxxx xxxx xxxx xxxx xxxxx Potent Cap.: 63 81 507 Move Cap.: 56 78 507 Volume/Cap: 0.00 0.00 0.00 0.02 0.00 0.08 0.03 xxxx xxxx xxxx xxxx xxxx _____ Level Of Service Module: B * * * * * * LOS by Move: * * * * * * Movement: LT - LTR - RT Shared LOS: * * * * C * * * * * * 22.0 XXXXXX XXXXXX xxxxxx ApproachDel: C Note: Queue reported is the number of cars per lane.

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Scenario Report

Scenario:

CUM PM

Command:

Volume:

Geometry:

Impact Fee:

Trip Generation:

Trip Distribution:

Paths:

Default Command

CUM PM

WB LEFT TURN

Default Impact Fee

TURN

Default Path

Default Path

Default Route

Routes:

Configuration: Default Configuration

Page 2-1

Trip Generation Report

Forecast for PM PEAK

Zone #	Subzone	Amount	Units	Rate In	Rate Out	Trips In	Trips Out	Total Trips	
1		9.10	RETAIL KSF	2.25	2.25	20	20	40	100.0
	Zone 1	Subtotal				20	20	40	100.0
TOTAL						. 20	20	40	100.0

Turning Movement Report

PM PEAK

Volume	Northbound		Southbound			Eastbound			Westbound			Total	
Type			Right			Right	Left	Thru	Right	Left	Thru	Right	Volume
#1 SR	29 / 1	Access											9 082
Base	0	0	0	0	0	1	0	1093	0	0	539	0	1633
Added	12	0	8	0	0	0	0	0	12	8	0	0	40
PassBy	4	0	8	0	0	0	0	8	-8	4	- 4	0	12
Total	16	0	16	0	0	1	0	1101	4	12	535	0	1685

Wed Feb 11, 2015 06:54:30

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CUMULATIVE PLUS PROJECT WITH TWLT LANE

MIDDLETOWN DOLLAR GENERAL 7240-06 ______

> Impact Analysis Report Level Of Service

Intersection

1 SR 29 / Access

Base Future Change
Del/ V/ Del/ V/ in
LOS Veh C LOS Veh C
B 12.1 0.002 D 28.3 0.107 +16.182 D/V

_____ Level Of Service Computation Report 2000 HCM Unsignalized Method (Base Volume Alternative) ************** Intersection #1 SR 29 / Access ************ Average Delay (sec/veh): 0.0 Worst Case Level Of Service: B[12.1] ************ Approach: North Bound South Bound East Bound West Bound Movement: L-T-R L-T-R L-T-RL - T - R Movement: L - T - R _____|___|___| Control: Stop Sign Stop Sign Uncontrolled Uncontrolled Rights: Include Include Include Rights: Include Include Include Lanes: 0 0 1! 0 0 0 0 0 0 1 1 0 0 1 0 1 0 0 1 0 Volume Module: 0 337 Base Vol: 0 0 0 0 1 0 683 0 Initial Bse: 0 0 0 0 0 1 0 1093 0 0 539 PHF Adj: PHF Volume: 0 0 0 0 0 1 0 1188 0 0 586
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0
FinalVolume: 0 0 0 0 0 1 0 1188 0 0 586 0 0 0 0 _____ Critical Gap Module: ______|__|__|__||___| Capacity Module: 586 XXXX XXXX XXXXX XXXX XXXX Cnflict Vol: 1774 1774 1188 xxxx xxxx Potent Cap.: 64 83 229 xxxx xxxx 510 xxxx xxxx xxxxx xxxx xxxx xxxx 510 xxxx xxxx xxxxx xxxx xxxx xxxx Move Cap.: 64
Total Cap: 168 83 229 xxxx xxxx Total Cap: Volume/Cap: 0.00 0.00 0.00 xxxx xxxx 0.00 xxxx xxxx xxxx xxxx xxxx xxxx xxxx _____|___| Level Of Service Module: 2Way95thQ: xxxx xxxx xxxx xxxx xxxx 0.0 XXXX XXXXX XXXXX XXXX XXXXX LOS by Move: * * * * * B * * * * * Movement: LT - LTR - RT 12.1 XXXXXX ApproachDel: XXXXXX В ApproachLOS: Note: Queue reported is the number of cars per lane. ***********

Level Of Service Computation Report 2000 HCM Unsignalized Method (Future Volume Alternative) ****************** Intersection #1 SR 29 / Access ************* Average Delay (sec/veh): 0.6 Worst Case Level Of Service; D[28.3] **************** North Bound South Bound East Bound West Bound L - T - R L - T - R Approach: L - T - R Movement: _____|___|___| Stop Sign Stop Sign Uncontrolled Uncontrolled Include Include Include Control: Rights: Include Includ Volume Module: Base Vol: 0 0 0 0 0 0 683 0 0 337 1 Tnitial Bse: 0 0 0 0 0 0 1 0 1093 0 0 539
Added Vol: 12 0 8 0 0 0 0 0 12 8 0
PasserByVol: 4 0 8 0 0 0 0 8 -8 4 -4
Initial Fut: 16 0 16 0 0 1 0 1101 4 12 535 PHF Volume: 17 0 17 0 0 1 0 1197 4 13 582
Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0
FinalVolume: 17 0 17 0 0 1 0 1197 4 13 582 _____| Critical Gap Module: _____[____[_____] Capacity Module: Cnflict Vol: 1807 1807 1199 xxxx xxxx 582 xxxx xxxx xxxxx 1201 xxxx xxxxx Potent Cap.: 61 79 226 xxxx xxxx 513 xxxx xxxx xxxx 581 xxxx xxxxx _____| Level Of Service Module: Control Del:xxxxx xxxxx xxxxx xxxxx xxxx 12.0 xxxxx xxxx xxxx 11.3 xxxx xxxxx LOS by Move: * * * * * B * * * *
Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT B * * LT - LTR - RT ApproachLOS: 28.3
ApproachLOS: D Note: Queue reported is the number of cars per lane.

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